## CORRIDOR Planning \＆Research

## Prepared For MDOT In Cooperation With The City Of St．Clair

Prepared by Spalding DeDecker Associates，Inc． 905 South Boulevard East Rochester Hills，MI 48307 January 10， 2005

# M-29 CORRIDOR RESEARCH AND PLANNING <br> MDOT CS 77052 JN 74272 

## FINAL REPORT

# Michigan Department of Transportation 

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# M-29 Corridor Planning and Research City of St. Clair, St. Clair County, Michigan 

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## Introduction and Background



## Introduction

M-29 is a state trunkline that runs from Macomb County easterly into St. Clair County, then heads north along the eastern shore of Michigan into the City of Marysville. With average daily traffic varying from 4,900 to 23,300 vehicles a day, the trunkline serves several different types of communities in different capacities along its length, with varying laneage and roadside features. Within the City of St. Clair, M-29 is primarily a four-lane roadway, with short portions of a two-lane roadway at the southern and northern City Limits. It is known locally as Oakland Avenue at the south end of the City and Riverside Avenue in the business district and north end of the City. Pedestrian access is limited to narrow sidewalks, which are not continuous along the corridor. It is bordered by residential, business and recreational property which has been well-established over the past century.

The Michigan Department of Transportation (MDOT), in cooperation with the City of St. Clair (City), has undertaken the planning and research of a non-motorized path along the M-29 corridor. This study also evaluates the laneage and operational features of the roadway for potential geometric modifications. Addressing both mobility needs and community needs, this study seeks to present opportunities to optimize the transportation and aesthetic features of the corridor benefiting motorists, pedestrians and bicyclists, alike.

## Introduction and Background

## Background - How we got here

The M-29 Corridor Planning Committee (CPC) was appointed by the St. Clair City Council in December of 2001 with the goal of planning future improvements for the M-29 roadway. Composed of resident volunteers and City and MDOT officials, the Committee sought to look at ways the M-29 facility could enhance the riverfront community. They have a vision which embraces the needs of the residents, business owners and visitors whose experiences in the city are affected by how the M-29 corridor operates.

M-29 is an MDOT-owned facility. Clearly, the City could not pursue its objectives without the knowledge, support and involvement of MDOT. The local MDOT Transportation Service Center (TSC) Manager participates on the M-29 CPC to advise on MDOT policies and standards which must be maintained in the corridor. Meeting throughout 2002, the M-29 CPC developed a list of objectives it sought to implement and investigated possible approaches for implementation.

Additional input was solicited by the M-29 CPC at Visioning Sessions held early in 2002, at which Focus Groups were asked to provided their opinions on what corridor features where important to them. The Focus Groups were organized by three geographic areas: North Riverside (M-29) , South Riverside/Oakland (M-29) and Downtown. Public input sessions were held February 11 and February 26, 2002, for the North and South Focus Groups, respectively. The Downtown Business owners' input was solicited via a survey, to which 18 responses were received by January 18, 2002. The input gathered from the Focus Groups was used in developing the overall objectives for this study.


With the TSC Manager's support, the M-29 CPC pursued MDOT Transportation Enhancement Program funding as a means to conduct the planning and research necessary for corridor improvements. The grant was awarded in the Fall of 2002. This study is the product of the administered Enhancement Grant.

## Objectives of the Planning and Research

## Study Objectives

With the leadership of MDOT's TSC and the insight of the M-29 Corridor Planning Committee, this study serves to provide recommendations toward setting and achieving the long-term vision for the corridor. A coordinated effort among City, County and State agencies will be necessary to develop and implement the goals. These goals, listed below, must be considered within the context of the corridor study limits and weighed against one another in order to promote a balanced approach in the development of corridor improvements.


- Plan for a continuous non-motorized path
- Reduce roadway noise
- Encourage motorists to obey posted speeds
- Improve safety at pedestrian crossings
- Improve turn movements, especially at Clinton Street
- Provide adequate parking on-street
- Improve aesthetics and suggest wayfinding signage and strategic placement of signs

Generally, the strategies and goals which MDOT sets for roadway improvements focus on safety, capacity (Level of Service) and ride quality. MDOT roadway rehabilitation projects undertake to meet these goals as a first priority. Additional supplemental corridor improvements are desirable, although many times the design schedule, budget or local participation necessary to implement those supplemental improvements are not available. The objectives set forth within this study are intended to provide recommendations for several desirable corridor improvements along M-29 which may be incorporated into future projects. This pro-active approach to providing early design input will allow the proper programming to take place and allow early consideration of these features which benefit both MDOT and the local community.

## Objectives of the Planning and Research

## Study Methods

To meet each of the objectives previously identified, the recommendations from this study have been developed in accordance with current MDOT, FHWA and AASHTO practices, guidelines, policies and standards. The traffic operation study which was completed as a part of this research was prepared in accordance with the Transportation Research Board Highway Capacity Manual and followed standard MDOT traffic study guidelines, as well.

To formulate the recommendations, the following items were investigated:

- M-29 right-of-way limits
- Existing topographic features
- Existing non-motorized paths and destinations in the community
- Alternate geometric configurations of the roadway
- Traffic counts and future use projections
- Existing and projected Levels of Service
- Potential landscaping opportunities
- Current and proposed land uses

In addition to completing a formal traffic study and associated geometric analysis of the corridor, public information meetings and a survey were conducted to gather public input regarding the study objectives. In November 2004, two public information meetings were held to allow comment and input on the Draft Study. Comments solicited at those meetings are summarized in Appendix B.

Previous reports and committee activities were reviewed to prevent duplication of effort and build upon what has already been accomplished. Alternative corridor roadway sections were developed in an effort to meet the study objectives and are illustrated in Section 5.

## Summary of Alternatives

Overall, four geometric alternatives of the roadway are presented herein. Each alternative meets the desired study objectives to differing degrees. An underlying objective inherent to all MDOT studies of this nature is maintaining an acceptable Level of Service for traffic using the facility.

How does each Alternate meet M-29 Corridor Study Objectives?

| Objective..... | Alternate 1 | Alternate 2 | Alternate 3 | $\begin{array}{l}\text { No-Build } \\ \text { Alternate }\end{array}$ |
| :--- | :--- | :--- | :--- | :--- |
| $\begin{array}{l}\text { Provide a } \\ \text { continuous non- } \\ \text { motorized path }\end{array}$ | $\begin{array}{l}\text { Alignment along Third } \\ \text { Street or along Palmer } \\ \text { Park, just outside of east } \\ \text { M-29 ROW line; bike } \\ \text { lane on shoulders north } \\ \text { and south of Business } \\ \text { District }\end{array}$ | $\begin{array}{l}\text { Alignment along and } \\ \text { within east side of M- } \\ \text { 29 ROW line; bike } \\ \text { lane on shoulders } \\ \text { north and south of } \\ \text { Business District }\end{array}$ | $\begin{array}{l}\text { Alignment along and } \\ \text { within east side of M- } \\ \text { 29 ROW line; bike } \\ \text { lane on shoulders } \\ \text { north and south of } \\ \text { Business District }\end{array}$ | $\begin{array}{l}\text { No change in } \\ \text { current non- } \\ \text { motorized access }\end{array}$ |
| $\begin{array}{l}\text { Reduce roadway } \\ \text { noise }\end{array}$ | $\begin{array}{l}\text { Addition of median } \\ \text { boulevard provides } \\ \text { landscaping and noise } \\ \text { abatement opportunities; }\end{array}$ | $\begin{array}{l}\text { Change from four to } \\ \text { three lanes will } \\ \text { encourage slower } \\ \text { speeds (reduces } \\ \text { roadway noise); } \\ \text { roadway geometry } \\ \text { encourages slower } \\ \text { speeds (reduces } \\ \text { landscaping will abate } \\ \text { roadway noise }\end{array}$ | $\begin{array}{l}\text { Change from four to } \\ \text { three lanes will to } \\ \text { encourage slower } \\ \text { speeds (reduces }\end{array}$ | $\begin{array}{l}\text { roadway noise); minor } \\ \text { opportunities for } \\ \text { landscaping will abate } \\ \text { roadway noise }\end{array}$ | \(\left.\begin{array}{l}Minor opportunities <br>

for landscaping are <br>
on east side of <br>
roadway, offering <br>
minor noise <br>
abatement <br>
benefits\end{array}\right]\)

Table 1


## Existing Area Profile

## Traffic Characteristics

Within the St. Clair City Limits, M-29 is 2.43 miles long, primarily a four-lane, undivided highway, with parking allowed on-street in the immediate downtown busines area. There exist 128 parking spaces on $\mathrm{M}-29$ in the downtown area. Cross streets and driveways intersect $\mathrm{M}-29$ throughout the corridor. In the immediate downtown area, however, all cross streets and most driveways exist on the west side of the roadway, as riverfront Palmer Park abuts the east side of the roadway. Additionally, a lift bridge and a railroad crossing exist just south of the Clinton Street intersection, periodically impacting traffic progression at the south end of the downtown business area.

Traffic volume within the project limits is approximately 15,000 vehicles per day, with less than 2\% commercial volume. The current Level of Service* (LOS) is at "B" or better within the fourlane section of $\mathrm{M}-29$, including the overall operation of the Clinton Street intersection. At the southern limits of the study area, $\mathrm{M}-29$ is a two-lane roadway operating with LOS "E". The two-lane roadway just north of the North City Limits is also currently operating at a LOS "E". These northern and southern LOS ratings are found in MDOT's 2000 Sufficiency Rating report.
*Level of Service is a description of the relative density of traffic, with "A' being the highest level of service and "F" representing the lowest level of service. Refer to Appendix A, Section 2.3 for further details regarding Level of Service.

# Existing Area Profile 

## Current Land Use and Plan

There are generally four types of land uses within the project study limits:

- Residential
- Commercial
- Industrial
- Recreational

The commercial use is focused primarily in the downtown business area. The residential areas lie north and south of the downtown area. A few industrial parcels exist immediately south of the Pine River lift bridge and the recreational area (Palmer Park) is on the east side of M -29 in the downtown business area. One vacant/agricultural parcel exists in the downtown business area, as well. See Figure 1 for a complete map of existing land uses.

The City's Community Comprehensive Plan does not indicate a significant change with regard to the $\mathrm{M}-29$ corridor. New commercial development is anticipated west of this study area, along Fred Moore Highway and Carney Drive. The existing high level of development along this segment of $\mathrm{M}-29$ is expected to remain the same, with redevelopment encouraging similar land uses. The nature of commercial redevelopment in the downtown business district is intended to provide a mix of needed retail services, promote local opportunities and encourage pedestrian access and activity in the business area.

## Right of Way

The public right-of-way width along the M-29 corridor varies from sixty (60) feet to one hundred and twenty (120) feet within the project limits. A field check of observed property boundaries was conducted and identified apparent conflicts compared to the right-of-way limits depicted on City tax maps (Figures $2 \mathrm{a}-\mathrm{c}$ ) as well as MDOT right-of-way maps (Figures $3 \mathrm{a}-\mathrm{c}$ ). The discrepancies in right-of-way do not alter the roadway recommendations herein, but should be verified and documented with a complete property boundary survey prior to beginning any final engineering design, as it will affect the development of roadside features.

Although not currently identified as a Scenic Heritage Route, the City is encouraged to apply for this corridor designation. If designated, the local community could then promote the route and its corridor to enhance tourism. Signs will be installed to identify the distinctive characteristics of the Heritage Routes, linking recreational or cultural features with a common theme. Additionally, future editions of Michigan's official map will identify the Heritage Routes.







## Alternative M-29 Roadway Sections

## DOWNTOWN BUSINESS AREA - Clinton Avenue to Vine Street

Keeping in mind the various objectives of the study, several potential roadway cross sections were evaluated. The following elements were considered as the roadway concepts were developed:

- Number of lanes
- Roadway geometrics
- Non-motorized path alignment
- Opportunities for aesthetic improvements
- Lighting
- Width of lanes
- Pedestrian crosswalks
- Parking
- Right-of-way
- Level of Service (LOS)

Of the several concepts investigated, four alternatives, including the "No-Build" alternative, were analyzed in detail. The Traffic Analysis Report evaluates all four alternatives under both current and future (2025) traffic conditions. and graphic representations were developed to clearly illustrate each of the four concepts (Figures 4-7).

When considering the LOS for each alternative, it is important to realize that the LOS is determined by a different factor for a two-lane highway than for a four-lane highway, and as such, describes different service conditions. That is, for a two-lane highway, the LOS is described as a function of the Percent Time Spent Following, or P.T.S.F., directly affecting the free-flow of a vehicle. For a multi-lane, or four-lane, highway, the LOS is described as a function of traffic volume compared to the highway's capacity, directly affecting a vehicle's ability to maneuver among other vehicles. A more detailed explanation of LOS analysis can be found in Appendix A, Section 2.3.

## No-Build Alternative

A No-build Alternative is presented as the alternative that does not permanently alter any features within the roadway. However, it does not necessarily preclude the City of St. Clair from pursuing roadside enhancements outside of the travelled roadway. Items such as decorative lighting, walkways or bike paths are viable additions to the corridor. The Nobuild Alternative maintains 128 parking spaces, but offers little opportunity to add landscaping. Based on the Speed Study performed in July 2002, many drivers are comfortable driving 10 mph over the posted speed limits with the roadway geometry the way it currently exists.

## Alternative 1- (Figures 4 \& 7)

This alternative maintains four lanes of through traffic as well as two parking lanes on either side of the road way, providing 130 parking spaces. With four through lanes, it maintains the same LOS as the No-build Alternative. The northbound and southbound


EXISTING M-29 TYPICAL CROSS-SECTION
(NO-BUILD ALTERNATE)


PROPOSED M-29 TYPICAL CROSS-SECTION
(ALTERNATE 1)


EXISTING M-29 TYPICAL CROSS-SECTION (NO-BUILD ALTERNATE )


PROPOSED M-29 TYPICAL CROSS-SECTION (ALTERNATE 2)


EXISTING M-29 TYPICAL CROSS-SECTION (NO-BUILD ALTERNATE )


[^0]Figure 7


## Alternative 1



Alternative 2


Alternative 3


PROPOSED M-29: 3-LANE VERSION ALTERNATE
NORTH \& SOUTH OF DOWNTOWN BUSINESS AREA OUT TOWARDS CITY LIMITS

## Figure 9



## Proposed Laneage North of Downtown St. Clair

M-29, North of Vine St.

## Alternative M-29 Roadway Sections

lanes are divided by a 16-foot median island, however, the non-motorized path must be located outside of the $\mathrm{M}-29$ right of way for this alternative. Along the west right of way line, only a concrete sidewalk will be provided. This alternative has the benefits of:

- Providing a median refuge for pedestrians crossing M-29 (crossing 2 traffic lanes)
- Creating visual interest within the wide pavement area
- Creating a sense of "narrowness" to encourage slower traffic
- Providing landscaping opportunities and buffers to roadway noise
- Providing space for low-level decorative lighting


## Alternative 2 (Figures 5 \& 7)

This alternative maintains two through lanes, creates a center left turn lane and maintains parking lanes on both sides of the roadway, providing 136 parking spaces. There is room within the existing right of way to include a non-motorize path on the east side of the roadway, as well as add a greenbelt along the west right of way line. The projected LOS for this alternative is lower than that of Alternative 1 or the No-build alternative. It should be noted that the accepted two-lane highway methodolgy that was used for the LOS analysis does not take into account the delay reduction from the center turn lane in a three-lane section. The analysis, therefore, yields a very conservative LOS. Actual conditions utilizing the center left turn lane would likely improve the LOS in the three-lane section. (See Appendix A, Section 3.3.2 for further details regarding traffic analysis methodology.) Furthermore, a decrease in the estimated traffic growth rate could result in minor changes to the LOS for this alternative. That is, changes in the regional traffic patterns, such as changes caused by the establishment of other north-south travel routes within the County, could affect such a decrease in the estimated growth rate. Actual development patterns within the region over the coming years should be monitored to verify if the projected traffic growth rate is realized. This alternative has the benefits of:

- Narrowing pedestrian crosswalks (crossing 3 traffic lanes)
- Creating a sense of "narrowness" to encourage slower traffic
- Creating a greenbelt on west side of roadway for landscaping
- Providing space for a bike path between east curb line and east right of way line


## Alternative 3 (Figures 6 \& 7)

This alternative maintains two through lanes, creates a center left turn lane and provides angled parking separated by a raised island along the east side of the roadway. 101 parking spaces are provided., however, there are minimal opportunities for landscaping or greenspaces within the right of way. A non-motorized path is included on the east side of the right of way, but only a sidewalk area is provided along the west right of way line immediately in front of the businesses. Like Alternative 2, Alternative 3 has a lower projected LOS than Alternative 1 or the no-build alternative. As described for Alternative

## Alternative M-29 Roadway Sections

2, assumptions regarding the benefit of the center turn lane as well as the estimated growth rate can be made here, which may result in actual decreases in the LOS being very minor. This alternative has the benefits of:

- Narrowing pedestrian crosswalks (crossing 3 traffic lanes)
- Creating a sense of "narrowness" to encourage slower traffic
- Creating a space for a bike path between east curb line and the east right of way line
- Increases parking spaces adjacent to Palmer Park


## SOUTH AND NORTH of DOWNTOWN BUSINESS AREA to CITY LIMITS

## Converting four-lane roadway to three-lane roadway (Figures 8 \& 9)

M-29 outside of the downtown business area is primarily a four lane roadway, with no onstreet parking (except for the short segment between Vine Street and Brown Street). This four-lane roadway section continues southerly and northerly toward the City Limits, where it transitions to a two-lane roadway near or at the City Limits. As a traffic calming measure and to provide bike lanes, the M-29 Corridor Planning Committee has proposed that these segments of $\mathrm{M}-29$ south and north of the downtown business area be converted from a four-lane roadway to a three-lane roadway, which includes a continuous center left turn lane and two six-foot bike lanes on each side of the through-lanes. The existing curb lines or shoulders would not be affected.

With the roadway outside of the City Limits being a two-lane roadway, it is reasonable to assume that the proposed conversion to a three-lane roadway within the City Limits would provide a better LOS utilizing a center turn lane than it's adjacent two-lane neighbor. However, standard highway capacity analysis methodology is based only on the number of through lanes available and does not account for the potential benefits of a center turn lane. Therefore, the LOS for the proposed three-lane section is considered the same as the existing two-lane roadway.

The three-lane conversion can be accomplished short-term or temporarily by modifying pavement markings, since neither the curb nor shoulder alignments are affected. For long-term conversion, the joint lines should be paved to be in alignment with lane lines.

## Alternative M-29 Roadway Sections

## CLINTON AVENUE INTERSECTION

Clinton Avenue is an east-west road which forms a T-intersection with M-29, just north of the Pine River. Its operation is complicated by the presence of the lift bridge (B2 of 77052) to the south and a commercial driveway to the east. Currently, the intersection operates at a high level of service (LOS "B" or better for all approach roads). Under the forecast conditions (2025), the intersection operates at with an overall LOS "B" or better, for all Alternatives, including the No-build Alternative. This is considered acceptable for any intersection on the State trunkline network.

In 2003 and 2004, the signal was modified during rehabilitation of the lift bridge, providing split phases with dedicated turn phasing in the intersection. Anecdotal reports suggest that traffic movements were improved using this modified signal operation. Further signal analysis to evaluate year-round operations should be conducted before permantly modifying the signal to incorporate new phasing.

However, given the location of the driveway to the Voyager Restaurant on the east side of the intersection, some drivers appear to experience confusion as to how best to exit the driveway and proceed north through the intersection. To provide a clearer view of the signal operation, a lower-height traffic signal could be mounted to the existing pole in the northwest quadrant of the intersection, angled to face northbound traffic. Also, a sign instructing drivers exiting the driveway to proceed through the intersection only on
 GREEN would be helpful.


Another feature of this intersection which may cause some drivers confusion while exiting the east driveway is the lighted signal face on the east side of the signal head. Although there is no roadway approach on the east side of the intersection, it is believed that the lighted signal facing east is there to assist pedestrians wishing to cross from the east side of $\mathrm{M}-29$ to the west. It is possible that drivers

## Alternative M-29 Roadway Sections

exiting the driveway on the east side get a glimpse of the signal face when it is in the green phase for Clinton Avenue, believing they have a green phase for M-29, instead. By providing pedestrian-activated crosswalk signal timing to this intersection, with pedestrian signals, the signal face on the east side of the signal head may be eliminated.

The span wire support system for the signals may be modified to mast arm supports to offer a moderate benefit in the placement of signal heads. The mast arm supports, in lieu of span wire support, would primarily offer an aesthetic benefit and would be considered too costly for the minor change in placement of signal heads it would afford.

## ACCESS MANAGEMENT

By managing the location, design and type of access to a parcel, good access management provides proven techniques to help reduce traffic congestion, preserve existing road capacity, improve traffic safety and reduce crashes. Currently, there are 18 points of property access along $\mathrm{M}-29$ in the downtown area with various spacing. Generally, the recommended spacing between access points is 185 feet for a 30 mph roadway.

Where possible, access to properties on the west side of the roadway should be provided via local cross streets. The biggest potential for access improvement exists on the east side of the roadway at the Voyager Restaurant. Relocating the driveway as far as possible from the intersection of Clinton Avenue is recommended. Because of the elevation difference between the roadway and the parking lot, this may result in a loss of parking spaces due to embankment that would need to be placed to fill in a driveway access west of the existing driveway.

At the St. Clair Inn, five access points currently exist for the property. Although access is limited due to the proximity of the river to the east, providing pass-through access between the driveways on the property may eliminate unneccesary vehicle maneuvers onto M-29 as drivers negotiate the various access decision points.

The absence of many driveways on the east side of the roadway benefits the flow of northbound traffic significantly. Should the City wish to request access points along the east side of the roadway for any future developments, careful consideration should be given to the placement and associated impacts fo such access.

Regardless of which roadway alternative is implemented, access management principles should be incorporated with any redevelopment efforts. Several effective design techniques are outlined in the 2001 Michigan Department of Transportation "The Access Management Guidebook" and is a recommended reference for any future design plans. Although not currently part of the City's Community Comprehensive Plan, access

## Alternative M-29 Roadway Sections

management principles must be recognized and supported by the City as well as MDOT in order to be effective. Amending the City's Plan to adopt an M-29 corridor overlay district establishing access management standards would be helpful in implementing the principles on any future redevelopment.

## AESTHETIC IMPROVEMENTS

Opportunities for additional landscaping improvements within the M-29 right of way are most prevalent in roadway Alternatives 1 and 2. The most visible planting opportunities exist in the median proposed in Alternative 1. Assuming a speed limit of $30-35 \mathrm{mph}$, both small and large plantings are possible. The actual layout of any median planting should be coordinated with plantings proposed for the Palmer Park improvements to create a uniform, cohesive image in the downtown riverfront area. Recommend plant species include, but are not limited to:

## Large Deciduous Trees

- Oaks
- Hackberry
- Birch
- Honeylocust
- Hickory

Ornamental Deciduous Trees

- Amelanchier
- Dogwood
- Hornbeam

Evergreens

- Pine

Large Shrubs

- Dogwood
- Viburnum
- Ninebark

Small Shrubs

- Euonymus
- Holly
- Potentilla
- Currant
- Viburnum
- Yew
- Juniper
- Hard Maples
- Sycamore
- Beech
- Ironwood
- Blue Beech
- Redbud
- Hawthorn
- Juniper
- Sumac
- Witchhazel

Other plant materials may be considered, however the species listed here are native to Michigan and would likely require the least maintenance to thrive. Watering and

## Alternative M-29 Roadway Sections

maintenance of any aesthetic plantings would be the responsibility of the City. The actual location of plantings must not interfere with clear vision requirements for vehicles and pedestrians, especially at driveways and intersections. Plantings within the M-29 corridor should be specifically located during the design phase. The addition of trees and shrubs within the corridor not only add visual interest, but also provide natural sound abatement.

Standard traffic regulatory and warning signs along the corridor are installed and maintained by MDOT. Other signing of local interest, such as local street signs and points of interest must be erected and maintained by the City and may require an MDOT permit. Other signs visible from the corridor but not related to traffic are regulated by City ordinances. Guidelines for placing new or replacing existing signs should assist property and business owners in the design and placement of their signs. Generally, signs should be simple in design and with a succint message. Where possible, signs should be consolidated to reduce clutter and assist the reader. Materials for non-MDOT signs may be regulated to require a particular type of material or color, as the City deems appropriate. Signs with flashing or moving parts are not recommended. Overall, the City should strive to maintain a uniform look to any local signs placed.

Decorative street lighting is recommended to replace the existing high mast luminares currently along the roadway. This improvement would be the City's responsibility to fund or secure funding from other sources.

> Example of building façade, signing, plantings and decorative street lighting coordinated through redevelopment effort in Romeo, Michigan.


## Alignment of Non-motorized Path

The National Center for Bicycling and Walking reports that across the country, bicycle and pedestrian tourism is making significant contributions to local economies. Studies show that where bicycle and pedestrian tourism is fostered and promoted, and where investments are made in bicycle and pedestrian facilities, the economic impact may be even greater. A thriving tourist industry, in turn, can attract and revitalize businesses, create jobs, and increase public revenue. This is precisely the driving force behind the development of trailways within Michigan, such as the Bridge to Bay Trail in St. Clair County.

Furthermore, more communities are recognizing that the development of bicycle and pedestrian facilities has a positive effect on nearby properties through which they pass. Homebuyers and business owners are realizing the value that such facilities bring to a community.

According to research conducted by Rails to Trails Conservancy, 85 million people used rail trails in 1994 alone. Given these numbers, it is easy to understand how communities can profit by responding to trail users' needs. Indeed, many types of businesses including restaurants, convenience stores, bicycle shops, campgrounds and bed and breakfast establishments - attribute at least part of their success to a nearby trail.

Locally and nationally, bicycle and pedestrian facilities have proven to be a cost effective use of public funds. ${ }^{1}$ It is important to note, however, that the design requirements (specifically, width) for a non-motorized path may vary depending on the source of funding. For this reason, potential non-motorized path alignments being evaluated for this corridor will assume the most conservative requirement of ten (10) foot paved width, plus two feet clear distance on either side of the path.

## Existing Paths and Plans in the Community

The Bridge to Bay Trail in St. Clair County is intended to pass through the City of St. Clair. The existing paved path along Fred Moore Highway and Carney Drive in the western part of the City is a part of this County-wide trail system. However, it currently does not complete the network, nor does it provide a continuous path through the City.

In 1997, the City of St. Clair completed a bicycle plan which summarized design standards and existing conditions for potential bicycle routes throughout the City. Regarding a path along the $\mathrm{M}-29$ corridor, the recommendation at that time was to construct a 12 foot path through Palmer Park along the River and through the Park, continuing north as an 8 foot sidewalk to Brown Street., with a crossing to the west at Brown Street. Heading south from Clinton Avenue and north from Brown Street, the recommendation was to convert the

## Alignment of Non-motorized Path

four lane M-29 roadway to a three lane roadway and create bike lanes on both sides of the road.

The recommendations made in 1997 sought to create a bicycle route network which would tie into existing paths and encompass popular community destinations. Links in the routes were provided along residential streets to tie into the $\mathrm{M}-29$ corridor. Those links recommended in 1997 are still valid today, however, the recommended placement of the path within the M-29 corridor is changed.

## Along M-29, From Clinton Avenue to Vine Street (Downtown Business Area)

Because of the anticipated aesthetic improvements planned within Palmer Park, the alignment of the non-motorized path can no longer be planned as a continuous path along the boardwalk or within the Park, as recommended in 1997. The design concepts for the beautification of Palmer Park include a linear fountain and pavilion which conflicts with the alignment of a path near the boardwalk along the St. Clair River or within the Park.

Roadway Alternatives 2 and 3 (Figures 5 and 6) presented herein provide space within the $\mathrm{M}-29$ right of way for a ten (10) foot paved path along the east side curb line, adjacent to Palmer Park. The path in this location would require the removal of several trees to keep the alignment within $\mathrm{M}-29$ right of way. The path could be designed to meander around the trees, but would require dedication of property within Palmer Park for this use.

Roadway Alternative 1 (Figure 4) presented herein does not provide a path within the M 29 right of way. Because Alternative 1 maintains four lanes of traffic, includes on-street parking on both east and west sides of the roadway and includes a raised median, there is not enough room within the existing right of way to include a non-motorized path, as well. There are two potential path alignments outside of the $\mathrm{M}-29$ right of way to consider: an alignment on Third Street or an alignment just inside the City Park property, immediately adjacent to the M-29 east right of way line.

Third Street, one block west of M-29, has 70 feet of roadway right of way and existing continuous sidewalk on the west side of the road. With lower traffic volumes and speeds than $\mathrm{M}-29$, as well as adequate right of way, a shared lane or a separate path is viable, from Clinton Avenue to Vine or Brown Street. (see Figure 10) The path would link back to M-29 following the same recommendations set forth in the 1997 Bicycle Plan. This alignment keeps the path on the west side of $\mathrm{M}-29$ and does not require the path to cross M-29.

## Alignment of Non-motorized Path

For a path alignment on City property adjacent to $\mathrm{M}-29$, the west edge of Palmer Park would be designated for the path. This may require the removal of four mature trees, if the 14 -foot clear width is required for the path. Two crossings of M-29 would be necessary to continue the path northerly and southerly to the City Limits. Crossings are recommended at Clinton Avenue and Vine Street.

## Along M-29, From Clinton Avenue to South City Limit

Right of way in this segment varies from 60 feet to 100 feet wide, limiting a continuous path alignment outside of the existing four-lane roadway. In the Bicycle Plan of 1997, this portion of the path was to be created by converting the four-lane section of M-29 to a three-lane section and adding a designated bicycle lane. This approach remains feasible. Conversion from four-lanes to three-lanes can be accomplished using pavement markings, however, long-term conversion should include pavement overlay or reconstruction to align joint lines with lane lines.

Should a path outside of the M-29 right of way be considered, a viable path alignment in this segment begins on the west side of $\mathrm{M}-29$ at Clinton, heading southerly over the Pine River then veers westerly off of M-29 onto Riverside Avenue. The path continues southerly beyond the City Limit into St. Clair Township. The local Riverside Avenue right of way is 100 feet wide, with lower traffic volumes and speeds than M-29. Either a shared lane or separate path alignment is possible.

## Along M-29, From Vine Street to North City limit

The M-29 right of way in this segment varies from 68 feet to 120 feet, limiting a continuous path alignment outside of the existing four-lane roadway. Existing right of way provides room for a path on the west side of the roadway near Vine Street, however, it can not accommodate a full 14 foot-wide path toward the northern end.

The Bicycle Plan of 1997 recommended converting this section of M-29 from a four-lane to a three-lane section, as well. This approach remains feasible and can be accomplished short-term with pavement markings, but long-term conversion should require paving to align joint lines with lane lines.

Another alignment, as supported by the Bridge to Bay Trail, follows Brown Street westerly to Range Road, then heads north out of the City to Davis Road before heading back easterly to M-29. This route traverses County roads and requires coordination with the St. Clair County Road Commission to pursue approval. The portion of this route on Brown Street may not accommodate a full-width path, however, and may require the relocation of utility poles.


## Alignment of Non-motorized Path

## Crosswalks

Regardless of the final alignment of the non-motorized path, pedestrian crosswalks will be maintained to provide safe crossings of $\mathrm{M}-29$ and intersecting local streets. Heightened delineation of all crosswalks can be accomplished by special emphasis pavement markings or by using differring pavement materials, such as brick pavers. Electronic systems to illuminate the crosswalk lines in the pavement are available, but are not currently approved for general use on State trunklines.


Special emphasis markings for crosswalks
Crosswalks are preferred at signalized intersections and stop-controlled intersections. Special consideration and delineation is required for crosswalks proposed elsewhere on M-29 and will be subject to a thorough safety analysis and approval by an MDOT Traffic and Safety Engineer.

## Implementation Plan

Action items for implementation can be categorized as either immediate (0-3 years) or future ( $3-10$ years) activities. Action items listed should be considered independent of one another and coordinated with related State and Local agency efforts. Estimated costs do not included administrative costs which may be incurred by State or Local agencies.

Comments solicited at the public information meeting held November 10, 2004, include a ranking of the corridor improvement objectives and are listed in Appendix B. Priority should be given to those action items for implementation which support the objectives ranked most important by the City and MDOT.

| Immediate Action Items <br> (Implementation 0-3 years) | Cost | Future Action Items <br> (Implementation 3-10 years) | Cost |
| :--- | :---: | :---: | :---: |
| Restriping lane lines to convert 4 <br> lane section to 3 lane section, north <br> and south of downtown business <br> area | $\$ 9,800$ | Construct Roadway Alternative 1 | $\$ 1,880,000$ |
| Landscape existing green spaces <br> Create uniform signing ordinance <br> for non-regulatory local signing | Varies | $\$ 0$ | Construct Roadway Alternative 2 <br> Construct Roadway Alternative 3 |
| Pursue Scenic Heritage Route <br> designation for M-29 corridor | $\$ 0$ | $\$ 1,549,000$ |  |
| Repave proposed north and south <br> Construct bike paths off M-29 <br> corridor (including engineering; not <br> including ROW costs) | with lane lines to match joint lines | $\$ 230,000$ |  |
| Pursue State and Federal grant <br> opportunities for Enhancements | $\$ 0$ | $\$ 500$ |  |
| Place special emphasis pavement <br> markings at crosswalks (Clinton | $\$ 5200$ |  |  |
| Ave. and at mall) |  |  |  |
| Modify signal at Clinton Avenue to <br> accommodate split-phasing | $\$ 18,000$ |  |  |

## Appendix A

## Traffic Analysis Report

# TRAFFIC ANALYSIS REPORT 

for the
M-29 CORRIDOR STUDY
in the
City of St. Clair, Michigan

PREPARED FOR:

Michigan Department of Transportation

AND


PREPARED BY:
PARSONS
December 2003

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## 1. INTRODUCTION

Parsons Transportation Group Inc. of Michigan (Parsons) has completed a traffic analysis study of the M-29 Corridor Study in the City of St. Clair. The limits of the study area extend between the south and north St. Clair city limits from approximately 500 feet north of Hatheway Street to approximately 2800 feet south of Yankee Road. The study area is illustrated on Figure 1. The purpose of the study was to assess the traffic and safety impacts of proposed alternatives for improving traffic operations, pedestrian crossings and safety, traffic safety, and parking and to control traffic speeds.

One of the alternatives includes reducing M-29 from 4 lanes with on-street parking to 3 lanes with a bike path and parking alternative in order to address traffic safety issues and improve the aesthetic appeal of the riverside $\mathrm{M}-29$ corridor. Although reducing lanes is not a common alternative, it is sometimes used as a traffic calming measure, and has been shown to reduce certain types of crashes according to a research report written by Michigan State University and approved by the Michigan Department of Transportation (MDOT) called "Guidelines for FourLane to Three-Lane Conversions".

This traffic analysis study for the M-29 Corridor Study in the City of St. Clair analyzed existing traffic conditions within the study area and future (2025) conditions for the No-Build and Build Alternatives. A traffic crash analysis was also completed for the project area using the most current data available as provided by the MDOT.

## 2. EXISTING CONDITIONS

### 2.1 Area Roadway Characteristics

A field review was conducted to collect data such as number of lanes and lane use, traffic control features (signal locations, speed limits, etc.) and other conditions necessary for the traffic analysis. Observations of traffic maneuvers were made, with particular attention to the drawbridge activity during peak traffic periods. The MDOT provided the signal timing plans for the M-29/Clinton Avenue intersection. The area road network is described in the following paragraphs:

M-29 is a north-south undivided highway having a 2-lane cross-section from the southern city limit (approximately 500 feet north of Hatheway Street) to Palmer Street, where it widens to a 4lane cross section with two lanes in each direction. M-29 remains a 4-lane cross section until the M-29/Clinton Avenue intersection, where north of Clinton, M-29 has a 4-lane cross section with a parking lane on each side from Clinton Avenue to approximately 500 feet north of Vine Street. Parking is prohibited on the west side of M-29 near Clinton Avenue to allow for an exclusive right-turn lane for southbound M-29 traffic at the Clinton Avenue intersection. M-29 transitions back to a 2-lane cross section at the northern city limit. The speed limit varies on M-29 from 40

miles per hour (mph) between the southern city limit and St. Clair Highway, to 35 mph from St. Clair Highway to Clinton, to 30 mph from Clinton to Brown, and returning to 40 mph from Brown to the northern city limit. Jay, Vine and Brown are local streets that are controlled by stop signs at their intersections with M-29 in the area of downtown St. Clair, the portion of the corridor immediately to the north of Clinton Avenue.

Clinton Avenue is an east-west road that forms a T-intersection with M-29. It has a two-lane cross section that widens to three lanes on the approach to the M-29 intersection. It has a left and right-turn only lane for eastbound traffic and one through lane for westbound traffic west of M29 and the speed limit is posted at 30 mph in this area.

The M-29/Clinton Avenue intersection is the only signalized intersection that exists along the approximately 2.5 mile stretch of M-29 study corridor between the St. Clair city limits. The signal is a pre-timed signal with bridge preemption. The signal has a 60 second cycle and operates in full color mode between the hours of 6 A.M. and 2 A.M., and operates in flash mode from 2 A.M. to 6 A.M. The bridge preemption is designated for time periods when the drawbridge is raised or being raised. When this occurs, the signal turns red for all movements on all approaches except the eastbound left-turns, which receives a green arrow; the eastbound right-turn movement receives a lighted case sign "No Right Turn" message. The drawbridge is opened on demand and at a maximum frequency of once every half hour during peak demand times in the summer. The drawbridge is raised for an average of 4 to 5 minutes at a time according to information found in "Summary of Bridge Opening Times for B02 of 77052, M-29 over the Pine River in the City of St. Clair 2001-2002 Data" by Spalding DeDecker Associates, Inc.

### 2.2 Existing Traffic Volumes

Twenty-four hour directional traffic volume counts on M-29 were performed from June 18, 2003 through June 25, 2003 between Jay Street and Vine Street and on June 19, 2003 at approximately 1,050 feet south of the drawbridge located between Clinton and Fort Street. The twenty-four hour directional traffic volume counts collected on Thursday, June 19, 2003 in both locations on M-29 were used for the base 2003 average daily traffic, and the directional volumes are shown on Figure 2. Other traffic volume data was available from the MDOT "Vehicle Classification Reports", the MDOT "Traffic Monitoring Information" (which are hourly volume count reports), the St. Clair Police Department, and the MDOT "Historic Annual Average 24 Hour Traffic Volume Maps". The collected traffic volume data was used to analyze traffic volume trends on M-29 with respect to locations north and south of the drawbridge, and by year, month and day of the week. These volumes were used as the basis for the existing traffic analysis portion of this study, including the capacity analysis of the M-29/Clinton Avenue intersection. All traffic volume count information completed by the MDOT is contained in Appendix I, and all traffic volume count information completed by Parsons is contained in Appendix II. Anomalies in the City's traffic counts precluded their use.

At the M-29/Clinton Avenue intersection, turning movement counts were performed between the hours of 7A.M. and 9A.M. and from 3P.M. to 5P.M. on Thursday, June 19, 2003. The hours between 7:00 A.M. and 8:00 A.M. and 4:00 P.M. and 5:00 P.M. were used as the A.M. and P.M.


LEGEND


BASE 2003 AVERAGE DAILY TRAFFIC VOLUMES
peak hours, respectively for determining base 2003 turning movement volumes. The detailed turning movement counts for the A.M. and P.M. peak hours are shown on Figure 3, and the data is contained in Appendix III.

The MDOT "Vehicle Classification Reports" were used to analyze the percent trucks using the M-29 corridor. The "Vehicle Classification Reports" contain 24 hour counts distinguishing thirteen different types of vehicles. For the following analysis, only trucks with trailers were considered "trucks", while single unit trucks and passenger cars were both considered "cars". The "Vehicle Classification Reports" include data for a complete 24 hours taken on Wednesday, May 9, 2001 at a location 0.5 miles south-west of Yankee Road in St. Clair Township, and a complete 24 hours taken on Tuesday, June 4, 2002 at a location 0.5 miles north of Recor Road in E. China Township. Although the count taken north of Recor Road is not located along the study segment of M-29, it is a location just south of the study segment and aids in the estimation of the percent trucks in the area. The total number of trucks counted during the 24 hours was divided by the total number of vehicles from these reports. At the location 0.5 miles south-west of Yankee Road, the northbound traffic included $1.51 \%$ trucks and the southbound traffic included $1.83 \%$ trucks. At the location 0.5 miles north of Recor Road, the northbound traffic included $1.34 \%$ trucks and the southbound traffic included $0.80 \%$ trucks. The average of the four truck percentages is $1.37 \%$. Therefore, a reasonable truck percentage for the $\mathrm{M}-29$ corridor is $2 \%$.

### 2.3 Capacity Analysis

### 2.3.1 Intersection Capacity Analysis

The M-29/Clinton Avenue intersection was analyzed according to the methodologies published in the most recent edition of the Highway Capacity Manual. The analysis determined the "Level of Service" of the location for the existing conditions. Levels of service are expressed in a range from "A" through "F," with "A" being the highest level of service, and "F" representing the lowest level of service. Level of Service (LOS) is based on factors such as number and types of lanes, signal timing, traffic volumes, pedestrian activity, etc. Table 1 shows the thresholds for Levels of Service "A" through "F" for signalized intersections. Table 2 summarizes the capacity analysis results for the existing conditions. Copies of the capacity analysis worksheets are contained in Appendix IV.

Table 1

## LEVEL OF SERVICE CRITERIA FOR SIGNALIZED INTERSECTIONS

| Level of Service | Delay/Vehicle <br> $($ seconds $)$ | Description |
| :---: | :---: | :--- |
| A | $<10.0$ | Most vehicles do not stop at all. |
| B | 10.1 to 20.0 | Some vehicles stop. <br> C |
| D | 20.1 to 35.0 | The number of vehicles stopping is significant, although many pass <br> through without stopping. |
| E | 35.1 to 55.0 | Many vehicles stop. Individual cycle failures are noticeable. <br> Considered to be the limit of acceptable delay. Individual cycle |
| F | 55.1 to 80.0 | failures are frequent. |
| SOURCE: Transportation Research Board, Highway Capacity Manual, Special Report 209,2000. |  |  |



Table 2
INTERSECTION LEVEL OF SERVICE-EXISTING CONDITIONS

| Intersection |  | AM Peak Hour |  | PM Peak Hour |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Approach | Delay | LOS | Delay | LOS |
|  | Overall | $\mathbf{8 . 1}$ | A | $\mathbf{1 0 . 3}$ | $\mathbf{B}$ |
|  | Southbound | 4.5 | A | 5.6 | A |
|  | Northbound | 8.4 | A | 10.2 | B |
|  | Eastbound | 15.6 | B | 18.5 | B |

It may be seen from Table 2 that all approaches during both time periods currently operate at high levels of service.

### 2.3.2 Segment Capacity Analysis

For purposes of calculating the segment capacities, M-29 within the St. Clair city limits was categorized into four segments depending on the road cross sections and speed limits. The segments were segregated in order to analyze the two-lane segment and the four-lane segments of M-29 separately. Their limits are described as follows: Segment 1 begins at the southern city limit ( $500^{\prime}$ N. of Hatheway) and ends at Palmer, Segment 2A begins at Palmer and ends at St. Clair Highway, Segment 2B begins at St. Clair Highway and ends at Clinton, Segment 3 begins at Clinton and ends at Brown (the "downtown" segment), and Segment 4 begins at Brown and ends at the northern city limit ( $2800^{\prime}$ S. of Yankee). Segment 2 had to be broken into two sections due to the change of posted speed limit from 40 mph to 35 mph at St . Clair Highway. The segments are divided similarly for the crash analysis section of this report.

The M-29 corridor between the St. Clair city limits was analyzed according to methods in the Highway Capacity Manual. Segment 1 is classified as a Class II two-lane highway segment; therefore it was analyzed using the two-lane highways methodology. Levels of service for Class II two-lane highway segments are defined in terms of average travel speed and percent time-spentfollowing and are expressed in a range from "A" through "F," with " A " being the highest level of service, and " $F$ " representing the lowest level of service. Table 3 shows the thresholds for Levels of Service "A" through " $F$ " for Class II two-lane highway segments.

Table 3
LEVEL OF SERVICE CRITERIA FOR CLASS II TWO-LANE HIGHWAY SEGMENTS

| Level of <br> Service | Percent Time-Spent- <br> Following |  |
| :---: | :---: | :--- |
| A | $\leq 40$ | Motorists are able to travel at their desired speed. |
| B | $>40-55$ | The demand for passing to maintain desired speeds becomes significant. |
| C | $>55-70$ | Noticeable increases in platoon formation, platoon size and frequency of <br> passing impediments. |
| D | $>70-85$ | Unstable traffic flow; passing becomes extremely difficult. |
| E | Flow Rate $>1,700 \mathrm{pc} / \mathrm{h}$ | Heavily congested flow with traffic demand exceeding capacity. |
| F |  |  |

SOURCE: Transportation Research Board, 2000 Highway Capacity Manual.
Segments 2, 3 and 4 are classified as multi-lane highway segments; therefore they were analyzed using the multi-lane highways methodology. Levels of service for multi-lane highway segments are defined in terms of density and are expressed in a range from "A" through "F," with "A" being the highest level of service, and "F" representing the lowest level of service. Density and Level of Service (LOS) are based on free-flow speed and flow rate, and density is measured as passenger cars per mile per lane ( $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ ). Table 4 shows the thresholds for Levels of Service "A" through "F" for multi-lane highway segments. Table 5 summarizes the capacity analysis results for the existing conditions.

Table 4
LEVEL OF SERVICE CRITERIA FOR MULTI-LANE HIGHWAY SEGMENTS

| Level of <br> Service | Density <br> $(\mathrm{pc} / \mathrm{mi} / \mathrm{ln})$ | $\leq 12.0$ |
| :---: | :---: | :--- |
| A | 12.1 to 20.0 | Description |
| B | 20.1 to 28.0 | Completely free-flow conditions. <br> Free-flow conditions with noticeable presence of other <br> vehicles. <br> Ability to maneuver is clearly affected by other <br> vehicles. |
| C | 28.1 to 34.0 | Ability to maneuver is severely restricted because of <br> traffic congestion. |
| D | $* 34.1$ to Volume-to-Capacity Ratio=1.0 | Operations at or near capacity; quite unstable. |
| E | Accurate prediction of density is difficult. | Highly unstable and variable traffic flow. |

[^1]Table 5
SEGMENT LEVELS OF SERVICE-EXISTING CONDITIONS

| M-29 Segment | Direction | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | P.T.S.F. ${ }^{(1)}$ | Density ${ }^{(2)}$ | LOS | P.T.S.F. ${ }^{(1)}$ | Density ${ }^{(2)}$ | LOS |
| Segment 1 - South City Limit (500' N. of Hatheway) to Palmer | NB | 100\% | - | E | 94\% | - | E |
|  | SB | 87\% | - | E | 94\% | - | E |
| Segment 2A - Palmer to St. Clair Highway | NB | - | 6.9 | A | - | 7.5 | A |
|  | SB | - | 4.4 | A | - | 8.0 | A |
| Segment 2B - St. Clair Highway to Clinton | NB | - | 7.8 | A | - | 8.5 | A |
|  | SB | - | 5.0 | A | - | 9.1 | A |
| Segment 3 - Clinton to Brown | NB | - | 6.5 | A | - | 11.1 | A |
|  | SB | - | 7.2 | A | - | 11.7 | A |
| Segment 4 - Brown to North City Limit (2800' S. of Yankee) | NB | - | 4.9 | A | - | 8.4 | A |
|  | SB | - | 5.5 | A | - | 8.9 | A |

(1) P.T.S.F. is an abbreviation for Percent Time Spent Following and is the determining factor of LOS for Class II two-lane highway segments.
(2) Density is the deternining factor of LOS for multi-lane highway segments.

It may be seen from Table 5 that the four-lane segments (Segments 2A, 2B, 3, and 4) during both time periods currently operate at high levels of service (LOS). Due to the lack of passing zones on the 2-lane segment (Segment 1), the percent time-spent-following is high and subsequently the LOS during both time periods is an " E ".

### 2.4 Speed Study Analysis

The MDOT Traffic and Safety Division conducted a speed study in July 2002 along the M-29 study corridor from 2000 feet south of Palmer Street to 400 feet north of Yankee Road. Along this stretch of M-29 the speed limit varies from a low of 30 mph to a high of 40 mph . The MDOT's speed study data is included in Appendix V. The provided data included the average speed and $85^{\text {th }}$ percentile for each of nine stations along the $\mathrm{M}-29$ study corridor. The speed at which $85 \%$ of the vehicles are traveling at or below is the 85 th percentile speed.

According to this study, the average speeds were all slightly higher than the posted speed limits, but none of the average speeds were greater than 6 mph over the speed limit. However, at two of the stations the $85^{\text {th }}$ percentile was found to be 10 mph over the speed limit. These stations were located near the intersections of M-29/Vine Street and M-29/Brown Street where the speed limit is posted at 30 mph . The $85^{\text {th }}$ percentiles reveal that drivers are comfortable driving at 10 mph over the posted speed limit near these locations. The speed limit may need to be evaluated at these locations to see if it is appropriate, bearing in mind that the current speed limits may need to be lower than the $85^{\text {th }}$ percentile due to pedestrian traffic or other factors. If any changes are made to $\mathrm{M}-29$ in terms of cross-section, lane configuration, etc. in the future, collecting new speed study data along the M-29 study corridor could be helpful in determining a new appropriate speed limit along M-29.

## 3. FUTURE CONDITIONS

### 3.1 Build Alternatives

Three alternatives are proposed for the reconfiguration of M-29 from approximately 250 feet south of Clinton Avenue to approximately 550 feet north of Vine Street. Two alternatives would provide a 10 foot wide bike path on the east side of M-29 and an 8 foot wide sidewalk on the west side.

Alternative 1 consists of the installation of a 16 foot wide island dividing two northbound through lanes and two southbound through lanes, with 10 foot wide parallel parking lanes on both sides of M-29. Crossovers would be provided for left-turning traffic entering and exiting the intersecting streets and select driveways. Alternatives 2 and 3 would convert the existing 4-lane section of M-29 to 3-lanes consisting of a through lane for northbound and southbound traffic and a center left-turn lane. Alternative 2 provides 10 foot wide parallel parking lanes on the east and west sides of M-29, while Alternative 3 provides a 28 foot wide angled parking area on the east side of M-29, separated from traffic on M-29 by a four foot divider island.

### 3.2 Forecast Traffic Volumes

Below is a summary of the development of the annual growth rate that was applied to the base traffic volumes in order to forecast traffic volumes for the design year 2025. Alone, none of the methods available were very accurate for calculating an annual growth rate; therefore, three different methods were used and analyzed to get a reasonable annual growth rate. Table 6 summarizes the annual growth rates calculated using the three different methods, and details on the methods follow the table.

Table 6
ANNUAL GROWTH RATE DEVELOPMENT METHODS COMPARISON


The first method used average daily traffic data from SEMCOG's regional travel demand model. SEMCOG's existing average daily traffic does not match the recently collected traffic volumes, and is lacking important details for the St. Clair area; therefore, in order for the model to give accurate results it would require a great deal of updated data. Also, it would need to be broken down into smaller zones in order to accurately model the St. Clair area. The large amount of additional data and analysis needed to make the model useable was beyond the scope of this study. However, the models forecasted traffic volumes were used to calculate an annual growth rate for M-29 just north and south of Clinton Avenue based on the difference between the models base year and forecast year directional ADT volumes.

The second method used the socioeconomic data from SEMCOG's regional travel demand model for six zones including the city of St. Clair's zone and the five surrounding zones. Annual growth rates were developed for the trips generated, population, number of households, and employment numbers based on the difference between the model's base year and forecast year numbers for the total of all six zones. This gave a good picture of the annual growth rate for many variables that influence average daily traffic volumes in the area.

The third method used data from MDOT's annually published "Annual Average 24 hour Traffic Volume" maps to calculate an annual growth rate of average daily traffic counts for the years 1989 through 2001. This historic count data was only available for two segments of M-29, from Clinton Avenue to Vine and from Vine to the north city limit. These annual growth rates represent only what has happened in the past, they don't take into account any other factors for predicting future traffic volumes.
After analyzing and reviewing all of the calculated annual growth rates, a reasonable growth rate of $1.25 \%$ for predicting future traffic on M-29 was determined. The annual growth rate of $1.25 \%$ was chosen, because most of the more applicable annual growth rates calculated were close to $1.25 \%$ and it is a more conservative rate than most of the other developed annual growth rates. Therefore, $1.25 \%$ is the annual growth rate that was applied to the base 2003 average daily traffic counts and A.M. and P.M. peak hour turning movements that were collected in June 2003, resulting in the forecasted traffic volumes for a design year of 2025. The forecast average daily traffic volumes are shown on Figure 4 and the forecast A.M. and P.M. peak hour turning movements are shown on Figure 5.

For a direct comparison of the capacity on M-29 under the existing and forecast conditions, it was assumed that the forecast traffic volumes for a no-build condition would be the same as forecast traffic volumes for a build condition, even though the traffic patterns may be slightly affected by the alternative selected for M-29. The forecast traffic is based on the same conditions and number of lanes as the existing conditions. Documented case studies have shown that conversions from 4-lane cross sections to 3-lane cross sections with a center left turn lane have resulted in a reduction of average or $85^{\text {th }}$ percentile speeds and larger reductions in excessive speeding; therefore, due to slower speeds resulting from a conversion to a 3-lane cross section, traffic volumes on M-29 may level off or increase at a slower rate than if M-29 was left as a 4lane cross section. Therefore, the assumption that traffic volumes would be the same for the nobuild condition as Alternatives 2 and 3 is a conservative one and is used for the resulting analysis.



FORECAST 2025 A.M./P.M. PEAK HOUR TURNING MOVEMENTS

### 3.3 Capacity Analysis

### 3.3.1 Intersection Capacity Analysis

The forecast conditions for the M-29/Clinton Avenue intersection were analyzed and the results summarized in Table 7. Copies of the capacity analysis worksheets are contained in Appendix VI.

Under forecast (2025) No-Build conditions, the M-29/Clinton Avenue intersection will continue to operate at an overall level of service (LOS) "A" during the morning peak hour and an overall LOS "B" in the afternoon peak hour with all approaches operating at a LOS of "C" or better. The intersection will also operate at an overall LOS "A" during the morning peak hour and an overall LOS " B " in the afternoon peak hour with all approaches operating at a LOS of "C" or better under forecast (2025) Build Alternative 1 conditions. Under forecast (2025) Build Alternative 2 conditions the intersection will operate at an overall LOS " $B$ " during the morning and afternoon peak hours with all approaches operating at a LOS of "C" or better. The intersection will also operate at an overall LOS "B" during the morning and afternoon peak hours with all approaches operating at a LOS of "C" or better under forecast (2025) Build Alternative 3 conditions.

Under forecast (2025) No-Build and all three Build Conditions, the M-29/Clinton Avenue intersection will operate at high levels of service; therefore mitigation measures did not need to be explored. However, the capacity analyses for forecast and existing conditions could not take into account any delays caused by traffic entering and exiting a driveway located on the east side of M-29, offset approximately 30 feet south of Clinton Avenue. The stop bar for northbound traffic is currently located just south of the driveway and is to remain in the same location for all three build alternatives. This helps to keep the driveway from being blocked, but field observations revealed southbound traffic intending to turn left into the driveway occasionally blocks southbound through traffic and causes longer delays than were able to be represented in the previously discussed capacity analyses.

### 3.3.2 Segment Capacity Analysis

For a direct comparison of the capacity on the four M-29 segments under the existing and forecast (2025) conditions, it was assumed that the forecast traffic volumes for a no-build condition would be the same as forecast traffic volumes for a build condition, even though the traffic patterns may be slightly affected by the alternative selected for M-29. Also, the free-flow speeds recorded on all four segments under existing conditions were used for the forecast conditions, because differences between the existing free-flow speeds and estimated forecast free-flow speeds can be expected to be minimal and it allows a direct comparison. The forecast conditions for the four M-29 segments were analyzed and the results summarized in Table 8.

Table 7
INTERSECTION LEVEL OF SERVICE - FORECAST 2025 CONDITIONS

| Intersection | Approach | No Build Condition |  |  |  | Alternative 1 |  |  |  | Alternative 2 |  |  |  | Alternative 3 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM Peak |  | PM Peak |  | AM Peak |  | PM Peak |  | AM Peak |  | PM Peak |  | AM Peak |  | PM Peak |  |
|  |  | Delay | LOS | Delay | LOS | Delay | LOS | Delay | LOS | Delay | LOS | Delay | LOS | Delay | LOS | Delay | LOS |
| M-29/ <br> Clinton Avenue | Overall | 8.7 | A | 12.4 | B | 8.3 | A | 11.1 | B | 10.8 | B | 17.9 | B | 10.8 | B | 17.9 | B |
|  | North | 4.8 | A | 6.1 | A | 4.7 - | A | 6.1 | A | 8.8 | A | 11.0 | B | 8.8 | A | 11.0 | B |
|  | South | 9.3 | A | 13.9 | B | 8.2 | A | 10.4 | B | 10.3 | B | 24.1 | C | 10.3 | B | 24.1 | C |
|  | West | 16.3 | B | 21.0 | C | 16.3 | B | 21.0 | C | 16.3 | B | 21.0 | C | 16.3 | B | 21.0 | C |

Table 8
SEGMENT LEVELS OF SERVICE-FORECAST 2025 CONDITIONS

| M-29 Segment | Alternative | Direction | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | P.T.S.F. ${ }^{(1)}$ | Density ${ }^{(2)}$ | LOS | P.T.S.F. ${ }^{(1)}$ | Density ${ }^{(2)}$ | LOS |
| Segment 1 - South City Limit (500' N. of Hatheway) to Palmer | No-Build | NB | 100\% | - | E | 95\% | - | E |
|  |  | SB | 90\% | - | E | 95\% | - | E |
| Segment 2A - Palmer to St. Clair Highway | No-Build | NB | - | 9.0 | A | - | 9.8 | A |
|  |  | SB | - | 5.9 | A | - | 10.5 | A |
| Segment 2B - St. Clair Highway to Clinton | No-Build | NB | - | 10.3 | A | - | 11.2 | A |
|  |  | SB | - | 6.7 | A | - | 11.9 | A |
| Segment 3 - Clinton to Brown | No-Build | NB | - | 8.4 | A | - | 14.7 | B |
|  |  | SB | - | 9.5 | A | - | 15.4 | B |
|  | Alternative 1 | NB | - | 8.4 | A | - | 14.7 | B |
|  |  | SB | - | 9.5 | A | - | 15.4 | B |
|  | Alternative 2 | NB | 91\% | - | E | 95\% | - | E |
|  |  | SB | 95\% | - | E | 96\% | - | E |
|  | Alternative 3 | NB | 91\% | - | E | 95\% | - | E |
|  |  | SB | 95\% | - | E | 96\% | - | E |
| Segment 4 - Brown to North City Limit (2800, S. of Yankee) | No-Build | NB | - | 6.4 | A | - | 11.1 | A |
|  |  | SB | - | 7.2 | A | - | 11.6 | A |

(1) P.T.S.F. is an abbreviation for Percent Time Spent Following and is the determining factor of LOS for Class II two-lane highway segments.
(2) Density is the determining factor of LOS for multi-lane highway segments.

The three proposed Build Alternatives under forecast conditions directly affect only Segment 3 (which is the "downtown" area); therefore only Segment 3 was analyzed under all three Build Alternatives and No-Build conditions. Build Alternative 1 does not change the number of through lanes in either direction from the No-Build conditions on Segment 3; therefore the results were the same as the No-Build conditions. However, Build Alternatives 2 and 3 both reduce the number of through lanes from two to one in both directions; therefore the results under Build Alternatives 2 and 3 were the same. However, realistically there will be less capacity for Alternative 2 due to parking/un-parking maneuvers, although the level of service methods cannot account for this situation.

Under the No-Build Alternative, both directions of travel on the four-lane segments (Segments $2 \mathrm{~A}, 2 \mathrm{~B}, 3$ and 4 ) will operate at levels of service (LOS) "A" during the morning peak hour and at LOS " $B$ " or better during the afternoon peak hour, while the two-lane segment (Segment 1) will continue to operate at LOS "E". Under Build Alternative 1, both directions of travel on Segment 3 will operate similarly to the No-Build Alternative; i.e., LOS "A" during the morning peak hour and LOS "B" during the afternoon peak hour.

Under Build Alternatives 2 and 3, both directions of travel on Segment 3 will operate at a LOS "E" during the morning and afternoon peak hours due to the inability to pass slower vehicles. The MDOT typically requires LOS "C" or better for planning purposes; however, it should be noted that the two-lane highway methodology that was used for the analysis gives a conservative
estimate of the LOS provided on Segment 3 under Build Alternatives 2 and 3. According to the Highway Capacity Manual 2000:
"There is no formal methodology for evaluating the traffic operational effectiveness of TWLTLs (Two Way Left Turn Lanes) on two-lane highways. Research has found that the delay reduction provided by a TWLTL depends on both the left-turn demand and the opposing traffic volume. Without a TWLTL or other left-turn treatment, vehicles that are slowing or stopped to make a left turn may create delays for following through vehicles. A TWLTL minimizes these delays and makes the roadway section operate more like two-way and directional segments with 100 percent no-passing zones... At higher-volume urban fringe sites, greater delay reduction was found with TWLTLs on a two-lane highway... As the delay reduction increases, a TWLTL can be justified for improving both traffic operation and safety."
So although the alternatives consisting of a three-lane cross section with a two way left turn lane (Build Alternatives 2 and 3) on Segment 3 had to be analyzed according to the two-lane highways methodology, this method does not take into account the delay reduction from the two way left turn lane. Nor does it take into consideration the delay increase due to parking/unparking maneuvers that would occur for Build Alternative 2. The levels of service provided on Segment 3 under Build Alternative 3 may actually be higher than LOS " $E$ ", but it is not expected it would move up more than one service level.

## 4. CRASH ANALYSIS

The crash history for the four-year period 1997 through 2000 was analyzed for the M-29 Corridor study area. The MDOT provided the crash data that was used to determine the critical crash locations within the study area. The crash data provided was for M-29 within the St. Clair city limits (from Hatheway Street to south of Yankee Road). The crash data received contained, by year, the total number of crashes, as well as a breakdown by the number of fatal, injury, and property damage only crashes. The data also included a breakdown of crash types by angle, rearend, sideswipe, backing, fixed object, animal, overturn, parking, bicycle, head-on, dual left turn, and other/unknown.

### 4.1 Crash Frequencies in the M-29 Study Area

For purposes of calculating the intersection crash frequency, crashes within 150 feet of the M29/Clinton Avenue intersection were considered to be intersection related crashes. For the fouryear period 1997 through 2000, there were a total of 154 crashes within the study area. Thirtyone occurred in 1997, 49 in 1998, 41 in 1999, and 33 in 2000, showing no definite increase or decrease during the study period.

For purposes of calculating the segment crash frequencies, all crashes along M-29 within the city limits were categorized into four segments depending on the locations and road cross sections. The segments are divided the same way as in the existing and future capacity analysis sections of this report. The segments were broken up in order to analyze the two two-lane segments and the two four-lane segments of M-29 north and south of Clinton separately to see what effect road
cross section and location north or south of Clinton had on the crash frequencies. Their boundaries are described as follows: Segment 1 begins at the southern city limit ( $500^{\prime} \mathrm{N}$. of Hatheway) and ends at Palmer, excluding crashes within 150' of Palmer, Segment 2 begins at Palmer and ends at Clinton, excluding crashes within 150' of Clinton, Segment 3 begins at Clinton and ends at Brown, excluding crashes within 150' of Brown, and Segment 4 begins at Brown and ends at the northern city limit ( $2800^{\prime}$ S. of Yankee).

Tables 9 and 10 provide summaries of crashes within the study area for the years 1997 through 2000. The crash frequency during each of the four years studied, the four-year total, and the average number of crashes per year are presented for the M-29/Clinton Avenue intersection and all segments in the study area.

Table 9
SUMMARY OF CRASH FREQUENCY FOR THE M-29/CLINTON AVENUE INTERSECTION

| Intersection Description | No. of Crashes by Year |  |  |  | $\begin{gathered} \text { 1997-2000 } \\ \text { Total Crashes } \end{gathered}$ | Annual Average Crash Frequency |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1997 | 1998 | 1999 | 2000 |  |  |
| M-29/Clinton Avenue Intersection | 7 | 4 | 2 | 4 | 17 | 4.25 |
| \% of Total | 41\% | 23.5\% | 12\% | 23.5\% | 100\% |  |

Table 10
SUMMARY OF CRASH FREQUENCY FOR SEGMENTS WITHIN THE STUDY AREA

| Segment Description | Mile Point ${ }^{(1)}$ |  | Length (Miles) | No. of Crashes by Year |  |  |  | $\begin{aligned} & 1997-2000 \\ & \text { Total } \\ & \text { Crashes } \end{aligned}$ | Annual <br> Average Crash Frequency |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | From | To |  | 1997 | 1998 | 1999 | 2000 |  |  |
| M-29 From S. City Limit (500' N. of Hatheway)To Palmer | 7.50 | 7.83 | 0.33 | 1 | 0 | 2 | 2 | 5 | 1.25 |
| M-29 From Palmer To Clinton | 7.83 | 8.54 | 0.71 | 11 | 22 | 23 | 13 | 69 | 17.25 |
| M-29 From Clinton To Brown | 8.54 | 9.26 | 0.72 | 16 | 21 | 11 | 15 | 63 | 15.75 |
| M-29 From Brown To N. City Limit (2800' S. of Yankee) | 9.26 | 9.99 | 0.73 | 3 | 6 | 5 | 3 | 17 | 4.25 |
| Total |  |  |  | 31 | 49 | 41 | 33 | 154 | 38.50 |
| \% of Total |  |  |  | 20\% | 32\% | 27\% | 21\% | 100\% |  |

(1) MDOT mile points run from south to north on M-29.

### 4.1.1 Intersection Crash Frequencies

Detailed crash breakdowns by type and severity for the M-29/Clinton Avenue intersection are presented in Table 11. A graphical representation of percentage by crash type for the M-

29/Clinton Avenue intersection is illustrated in Figure 6. The detailed crash listings can be found in Appendix VII.

Table 11
INTERSECTION CRASH DETAIL FOR THE YEARS 1997 THROUGH 2000

| Intersection | Total No. Of Crashes | Crash Type |  |  |  |  |  |  |  | Crash Severity |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AN | BIKE | MSC | PRKG | RE | SS | OT | HD | Fatal | Injury | PDO |
| M-29/Clinton Avenue | 17 | 2 | 2 | 2 | 2 | 5 | 2 | 1 | 1 | 0 | 6 | 11 |
| Percent of Total * |  | 12\% | 12\% | 12\% | 12\% | 29\% | 12\% | 6\% | 6\% | 0\% | 35\% | 65\% |

Abbreviations for Crash Type:
AN: Right Angle
BIKE: Bicycle
MSC: Crash type was coded improperly or not coded
PRKG: Parking
RE: Rear End
SS: Sideswipe/Same Direction

OT: Over Turn
HD: Head-On or Head-On Left Turn
Fatal: Crash that resulted in at least one fatality
Injury: Crash that resulted in at least one injury
PDO: Crash that resulted in property damage only (no injuries or fatalities)

* : Percentages do not add to $100 \%$ due to rounding

A review of crash severity as presented in Table 11 indicates that none of the crashes occurring at the $\mathrm{M}-29 /$ Clinton Avenue intersection during the four-year period involved a fatality. However, over one-third ( $35 \%$ or 6 crashes) of the 17 total intersection crashes involved an injury.


Figure 6: CRASH TYPE SUMMARY FOR M-29/CLINTON AVENUE INTERSECTION 1997-2000

A review of Table 11 and Figure 6 indicates that $29 \%$ of all crashes at the M-29/Clinton Avenue intersection were rear-end crashes, which makes them the most frequent type of crash during the four-year study period. Rear-end crashes are a common type of crash found at intersections, due to drivers starting and stopping for the traffic signal or stopped traffic. The M-29/Clinton Avenue intersection experienced a total of 17 crashes, 5 of which were rear-end crashes and the other seven types occurred either once or twice within the four-year period. There is not an unusually high incidence of any one type of crash.

### 4.1.2 Segment Crash Frequencies

Detailed crash breakdowns by type and severity for each segment are presented in Table 12. A graphical representation of percentage of crashes by crash types for all segments is illustrated on Figure 7. Detailed crash listings, which include mile point, crash date, crash ID, crash area, crash location, crash type, number of injuries and number of fatalities for each crash, can be found in Appendix VII.


Figure 7: CRASH TYPE SUMMARY FOR ALL STUDY SEGMENTS 1997-2000
A review of the crash severity as presented in Table 12 indicates no fatal crashes occurred on any of the study area segments during the four-year period. However, $34 \%$ ( 52 crashes) of the 154 total segment crashes involved an injury.

A review of Table 12 and Figure 7 indicate $32 \%$ of all segment crashes were rear-end crashes, and $32 \%$ were right angle crashes. The other nine types of crashes that occurred on the segments all occurred with much less frequency. On the segment between Palmer and Clinton Avenue it appears that many of the rear-end and right angle crashes are the result of uncontrolled access on M-29 in this area. Also, ten of the 31 angle crashes on this segment occurred at the intersection of M-29 and St. Clair Highway as the result of vehicles turning onto M-29 from St. Clair Highway. The rear-end and right angle crashes that occurred on the segments north of the bridge are likely the result of the downtown traffic becoming congested at certain times of the day causing unexpected stops, and also from turning movements in and out of the driveways on the west side of M-29. Also, four of the six over-turn crashes on the segment between Clinton Avenue and Brown occurred at the intersection of M-29 and Vine Street. With parking allowed intermittently along the east and west side of M-29 from Clinton Avenue to Vine Street, it is difficult for drivers to clearly see vehicles traveling north and south on M-29 before they exit driveways making right or left turns onto M-29.

Table 12
SEGMENT CRASH DETAIL FOR THE YEARS 1997 THROUGH 2000

| Segment Description | Mile Point |  | Length <br> (Miles) | Total \# Of Crashes | Crash Type |  |  |  |  |  |  |  |  |  |  |  | Crash Severity |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | From |  |  |  | AN | RE | BCKG | MSC | SS | FXOB | PRKG | BIKE | OT | HD | DU | ANML | Fatal | Injury | PDO |
| M-29 From S. City Limit (500' N of Hatheway)To Palmer | 7.50 | 7.83 | 0.33 | 5 | 2 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 2 |
| M-29 From Palmer To Clinton | 7.83 | 8.54 | 0.71 | 69 | 31 | 23 | 0 | 7 | 3 | 4 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 24 | 45 |
| M-29 From Clinton To Brown | 8.54 | 9.26 | 0.72 | 63 | 14 | 19 | 1 | 6 | 6 | 0 | 4 | 2 | 6 | 3 | 2 | 0 | 0 | 19 | 44 |
| M-29 From Brown To N. City 9.26 9.99 Limit (2800' S. of Yankee) |  |  | 0.73 | 17 | 3 | 6 | 0 | 0 | 3 | 2 | 0 | 0 | 0 | 1 | 0 | 2 | 0 | 6 | 11 |
| Totals |  |  | 2.49 | 154 | 50 | 50 | 2 | 13 | 12 | 6 | 5 | 2 | 6 | 4 | 2 | 2 | 0 | 52 | 102 |
| Percent of Total * |  |  |  |  | 32\% 32\% |  | 1\% | 8\% | 8\% | 4\% | 3\% | 1\% | 4\% | 3\% | 1\% | 1\% | 0\% | 34\% | 66\% |
| Abbreviations for Crash Types: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AN: Right Angle |  |  | OT: Over Turn |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| RE: Rear End |  |  | HD: Head-On |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| BCKG: Backing Up |  |  | DU: Dual Left-Tum |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| MSC: Crash type was coded improperly or not coded |  |  | ANML: Crash With Animal |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | Fatal: Crash that resulted in at least one fatalityInjury: Crash that resulted in at least one injury |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| FXOB: Fixed Object |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| PRKG: Parking |  |  | PDO: Crash that resulted in property damage only (no injuries or fatalities) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| BIKE: Bicycle |  |  | * : Percentages do not add to $100 \%$ due to rounding |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

### 4.2 Identification of High Crash Locations

Besides crash frequency, crash rates are developed and used to form a common base to allow comparisons from intersection to intersection or segment to segment. The crash rates used in this analysis were based on the number of crashes per million entering vehicles (MEV) for intersections and the number of crashes per million Vehicle Miles of Travel (MVT) for segments.

### 4.2.1 Intersection Crash Rate Analysis

The rate for the M-29/Clinton intersection was calculated based on the following formula:

$$
\text { Crash Rate }=\frac{\left(\text { Total No. Of Crashes } \div \text { No. Of Years) } \times 10^{6}\right.}{365 \times \text { Total Entering Volume (in vehicles per day) }}
$$

The total entering volume used was 19,272 vehicles per day and was approximated from the June 2003 average daily traffic counts and turning movement counts performed at the M-29/Clinton Avenue intersection. In order to compare the M-29/Clinton Avenue intersection crashes with regional averages, the SEMCOG Regional Segment and Intersection Analysis Report, conducted for 1988-1990 data was utilized. The intersection data of that report included 1,315 signalized intersections from different road classifications. The annual average crash frequency and rate in the SEMCOG Region is based on functional class, area and ADT volumes and the appropriate category was used for comparison. The results of the analysis are shown in Table 13.

Table 13
CRASH RATE SUMMARY FOR M-29/CLINTON AVENUE INTERSECTION

|  | Intersection <br> Daily Entering <br> Volume | Intersection <br> Annual Avg. <br> Crash Frequency | SEMCOG Region <br> (1) <br> Annual Avg. Crash <br> Intersection <br> Frequency | Annual <br> Crash Rate | SEMCOG <br> Region ${ }^{(1)}$ Annual <br> Avg. Crash Rate |
| :---: | :---: | :---: | :---: | :---: | :---: |
| M-29/Clinton Avenue | 19,272 | 4 | 11 | 0.60 | 1.82 |

${ }^{(1)}$ From SEMCOG Regional Segment and Intersection Analysis Report, 1988-1990 data,
The M-29/Clinton Avenue intersection's annual average frequency of crashes is less than half the region-wide annual average crash frequency. Also, the intersection's annual crash rate is 0.60 crashes per million entering vehicles (MEV), while the region-wide annual average crash rate is over twice as high. Therefore, it can be concluded that the M-29/Clinton Avenue intersection would be considered a lower crash location based on frequency and rate.

### 4.2.2 Segment Crash Rate Analysis

Crash rates were calculated for each study segment in the area based on the following formula:
Crash Rate $=\frac{\left(\text { Total No. Of Crashes } \div \text { No. Of Years) } \times 10^{6}\right.}{365 \times \text { ADT } \times \text { Segment Length in Miles }}$

The average daily traffic volumes (ADT) for the base year 2003 were used for the study segment crash rate calculations. The base year 2003 ADT from the June 2003 traffic volume counts taken at approximately 1,050 feet south of the drawbridge on M-29 were used for the segment of M-29 between the southern city limit and Palmer Street and the segment of M-29 between Palmer Street and Clinton Avenue. The base year 2003 ADT from the June 2003 traffic volume counts taken between Jay Street and Vine Street were used for the segment of M-29 between Clinton Avenue and Brown Street and the segment of M-29 from Brown Street to the northern city limit. In order to be able to compare the M-29 study segment's crashes with regional averages, the SEMCOG Regional Segment and Intersection Analysis Report conducted for 1988-1990 data was utilized. Although the data may appear dated, it is more comprehensive than more recent information available through other sources. The segment data of that report included 6,224 segment links from different road classifications. The results of the analysis are presented in Table 14, and the study area segments are sorted by descending crash rate. The annual crash rates in the SEMCOG Region, used for a comparison and found below in Table 14 are average rates calculated from given rates based on area type and averaged daily traffic (ADT), number of lanes and ADT, and functional classification and ADT.

Table 14
CRASH RATE SUMMARY AND COMPARISON FOR STUDY AREA SEGMENTS

| Segment Description | M-29 Area Study Segments |  |  | ```Annual Crash Rate In SEMCOG Region }\mp@subsup{}{}{(1)``` |
| :---: | :---: | :---: | :---: | :---: |
|  | ADT | Length (Miles) | Annual Crash Rate |  |
| M-29 From Palmer To Clinton | 14,694 | 0.71 | 4.53 | 6.08 |
| M-29 From Clinton To Brown | 15,122 | 0.72 | 3.96 | 6.08 |
| M-29 From Brown To N. City Limit (2800' S. of Yankee) | 15,122 | 0.73 | 1.05 | 5.27 |
| M-29 From S. City Limit (500' N. of Hatheway)To Palmer | 14,694 | 0.33 | 0.71 | 5.27 |

${ }^{(1)}$ From SEMCOG Regional Segment and Intersection Analysis Report, 1988-1990 data.
All of the study area segments had lower crash rates than the region averages, but the M-29 segment between Palmer and Clinton had the highest crash rate (4.53) of the four study segments. A review of the detail crash data of the M-29 segment between Palmer and Clinton indicates 69 crashes occurred during the four-year period in this segment, thirty-one of which were right angle crashes and twenty-three of which were rear end crashes. Although there are more right angle and rear end crashes than any other type of crash on this segment, it does not appear unusual. Many driveways exist along this segment, and left-turns to the numerous driveways have to be made from a through lane, which could be the cause of many of the rear end crashes.

## 5. CONCLUSIONS

Based on the results of the traffic and crash analysis performed for this study, the following conclusions can be drawn:

1. Currently the intersection of M-29/Clinton Avenue operates at overall acceptable levels of service (LOS) "A" during the morning peak hour and " $B$ " in the afternoon peak hour.
2. Currently both directions of travel on the four-lane M-29 segments (Segments 2A, 2B, 3, and 4 as described in Section 2.3.2) during both the morning and afternoon peak hours operate at LOS "A", while the one two-lane segment (Segment 1) operates at LOS "E" during both time periods.
3. Under forecast (2025) No-Build conditions, the M-29/Clinton Avenue intersection will continue to operate at an overall LOS "A" during the morning peak hour and an overall LOS "B" in the afternoon peak hour.
4. Under forecast (2025) conditions, the No-Build Alternative would allow both directions of travel on the four-lane segments (Segments 2A, 2B, 3 and 4) to operate at LOS "A" during the morning peak hour and LOS "B" or better during the afternoon peak hour, while the two-lane segment (Segment 1) will operate at LOS "E".
5. Under forecast (2025) Build Alternative 1 conditions, both directions of travel on Segment 3 (the "downtown" area) would operate at LOS "A" during the morning and LOS "B" during the afternoon peak hours.
6. Under forecast (2025) Build Alternative 2 and 3 conditions, both directions of travel on Segment 3 would operate at LOS "E" during the morning and afternoon peak hours due to the inability to pass and intense platooning of traffic. The MDOT typically requires LOS "C" or better for planning purposes.
7. Traffic crash data reviewed for the M-29/Clinton Avenue intersection between 1997 and 2000 indicated a total of 17 intersection related crashes. The notable findings from the information reviewed is as follows:

- Thirty-five percent (6 crashes) involved an injury.
- Twenty-nine percent of all crashes were rear-end crashes, resulting in the majority of crashes.
- The study intersection would not be considered a high crash location when compared with region wide intersection crash data.

8. Traffic crash data reviewed for the four-year period 1997 through 2000 indicated a total of 154 crashes along the M-29 study corridor, exclusive of the M-29/Clinton Avenue intersection. The notable findings from the segment crash information reviewed is as follows:

- The most common types of crashes occurring on the four M-29 segments were right-angle and rear-end crashes, each of which are responsible for thirty-two percent of all of the studied segment crashes.
- The largest incidence of crashes occurred on M-29 from Palmer to Clinton (69 crashes) and on M-29 from Clinton to Brown ( 63 crashes). These two segments are located in the "downtown" St. Clair area and they include the only signalized intersection along M-29 within the St. Clair city limits.
- None of the segments studied, however, would be considered high crash locations based on comparisons with road segments with similar features.


## APPENDIX I

 MDOT TRAFFIC VOLUME COUNT DATA


## 




## Michigan Department of i ranspuriauuu

Vehicle Classification Report




|  | CRs | MX | PK | BS | . 2 Ax | . $34 \times$ | 4Ax | TH | 4A8 | 5AI | 6AX | T | 5Ax | 6 Al | 7 Ax |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| TH1 | 4285 | 34 | 1150 | 54. | -35 | 12 | 1 | 102 | 48 | 32. | 5 | 85 | 0 | 0 | 7 |




TOTAL VEHICLES : 5663


Euiurty St, Claix .-. Cantrol Seetionn 77052 :
stat Mesc
Comastraction

Statamor - 78
Mriept: . . . - - 10:0ht
-•Praute \#

Count Type STATEWIDE
Track or Tractor with



$$
\text { TOTAL VEHCLES } \quad \text { G107 }
$$

# Michigan Ueparment or 1 ranspurvanum 

Vehicle Classification Report


## Stat Dese $\quad 0.5 \mathrm{MIN}$ OF RECOR RDIS OF HAWTIGORN-ECHINA TWP Ronte Desc M-2g

Construction
Count Type STATEWME
Truck or Tractor with



```
TOTALVEATCLES , 3884
```

[vichigan Lepartment of Iransportation
Vehicle Classification Report

Pr rante \# figanooo
Birectiou N Bat Desc 0.5 IUI NOF RECOR RD/S OF HAPWHORN-E CHONA TWP Constraction

Conut Type statewnoe
Route Bese M-20



## TOTAL VEHICAES 4000

# Hicngan yeparmmenu in inasportamun 

Velicle Classification Report



## Vehicle Classification Report

County St．Clair

Control Section 77052
Statmira Milept．
． 04.850
Route Desc－M－29

## Comstruction

Conut Type STATEWIDE

|  |  |  | Passeng |  |  |  | Unit | ueks |  |  |  |  | uck | Tra | with |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Thine | Date | Day | CRs | MX | PK | 6S | 2Ax | 3Ax | $44 x$ |  |  | gle Tit |  |  | Dou | c Trir |  |  |  |
| 14010 | $06 / 44 / 2002$ | TUE | 195 | 1 | 61. | 0 | 2 | 3AR | $4{ }^{4 \times}$ | Tti | 4 Ax | 5Ax | 6 A 6 | Tla | SAK | 6 dx | $74 x$ | Tol |  |
| 1510 | $06 / 04 / 2002$ | TUE | 221 | 0 | 51 | 0 | 3 | 0 | 0 | 3 | 2 | 1 | 0 | 3 | 0 |  |  | TH | $Y$ Hes |
| 1600 | 06／04／7002 | TME | 267 | 0 | 79 | 0 |  | 0 | （） | 3 | 2 | 0 | 0 | 2 | 0 |  | 0 | 0 | 263 |
| 1700 | 06／04 $/ 2002$ | TUE | 274 | 0 | 78 | 0 | 8 | 0 | 0 | 8 | 1 | 1 | 0 | 2 | － |  | 0 | ［ | 277 |
| 180月 |  | TUEE | 254 | 1 | 68 |  | 3 | 0 | 0 | 3 | 0 | 0 | 0 | a | 0 |  | 1 | 0 | 356 |
| 1904 | 06／04 $/ 21002$ | TUE | 177 | 0 | 40 | 0 | O | 0 | 0 | ． 0 | 1 | 0 | 0 | 1 | 0 |  | 0 | 0 | 355 |
| 2000 | 06／04／3002 | TUE | 144 | 0 | 30 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 目 | 0 |  | 0 | 0 | 324 |
| 2100 | $06.04 / 2012$ | TCE | 107 | 1 | 34 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 217 |
| 2200 | 106／04／2002 | TuE | 78 | 1 | 14 | 0 | 0 | 0 | ${ }^{1}$ | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 174 |
| 2300 | 166／04 12102 | TUE | 67 | 0 | 13 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 143 |
| 2400） | 06／04／21022 | TUE | 41 | 0 | 11 | 0 | 0 | 0 | 0 | ${ }^{6}$ | 0 | 0 | 1 | 0 | 0 | 0 | $\theta$ | 0 | 102 |
| ．1100 | $06 / 05 / 20412$ | Wed | 21 | 0 | 118 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | a | 0 | 0 | 80 |
| 0200 | 106／45／2002． | WED | 9 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | ， | 0 | f | 0 | 0 | 52 |
| 0300 | 06／05／2002 | WED | 5 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | a | $\theta$ | 0 | 1 | 10 | 30 |
| （14， $0_{0}$ | 06／05／2002 | WED | 5 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | O | 0 | 0 | 15 |
| 0 0N） | $06 / 05 / 2002$ | WED | 13 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | ， | 0 | 0 | 0 | 0 | 8 |
| 0660 | $06 / 05 / 2002$ | WeD | 52 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | f | 0 | 0 | 0 | 0 | 0 | 12 |
| 0700 | 06／05／2402 | Wed | 126 | 0 | 35 | 0 | 0 | 1 | 0 | 1 | 1 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 26 |
| 489 | 1／5／05／20 | Wen | 147 | 0 | 92 | 0 | 2 | 0 | 0 | 2 | 1 | 0 | 0 | 2 | 0 | 0 | 1 | 1 | 93 |
| 09\％ | $06 / 05 / 2002$ | WEE | 130 | 0 | 44 | 0 | 6 | 0 | 0 | 6 | 0 | 1 | 0 | 1 | 1 | 0 | 2 | 2 | 233 |
| 1000 | 06／05／2002 | Wued | 144 | 0 | 35 | 0 | 7 | 1 | 1 | 9 | 3 | 2 | 0 | 5 | 4 | 0 | 0 | 0 | 198 |
| 1100 | $06 / 05 / 2002$ | WED | 152 | 0 | 47 | 0 | 3 | 1 | 1 | 4 | 0 | 3 | 0 | 5 | 0 | 0 | 4 | 4 | 183 |
| 12000 | $06 / 05 / 2002$ | WYEO | 151 |  | 47 | 0 | 3 | 2 | 0 | 5 | 1 | 1 |  | 3 | 0 | 0 | 1 | I | 208 |
| 1360 | 06／05／2002 | WED | 191 | 0 | 73 | 0 | 5 | 0 | 0 | 5 | 1 | 0 | 1 | 2 | 0 | 0 | I | 1 | 207 |
|  |  |  |  |  | 49 | 0 | 3 | 2 | 0 | 5 | 1 | 0 |  | 2 | 0 | 0 | 0 | 0 | 231 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | 1 | 0 | 0 | 2 | 2 | 248 |



APPENDIX II PARSONS TRAFFIC VOLUME COUNT DATA

Traffic Data Collection, Inc.
Page 1
61891 Spring Circle, Washington MI. 48094
Traffic Study Performed For:
Project: St. Clair
Count Type: 24 Hr . ATR Volume Count Weather: Clear, Dry Counter\#:7105

Parsons Transportation Group, Inc.

Station ID: Southbound
RIVERSIDE (M-29)
(0.20 Mile South Drawbridge)

SBRiversideS1
Site Code: 0000003

Date Printed: 26-Jun-03



ADT Not Calculated

Traffic Data Collection, Inc.
61891 Spring Circle, Washington MI. 48094
Traffic Study Performed For:
Project: St. Clair
Count Type: 24 Hr . ATR Volume Count Weather: Clear, Dry Counter\#:7104

| Total | 0 | 0 | 1316 | 7262 | 0 | 7360 | 0 | 0 | 7360 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| \% Avg. WkDay | 0.0\% | 0.0\% | 17.9\% | 98.7\% | 0.0\% | 100.0\% |  |  |  |
| \% Avg. <br> Week | 0.0\% | 0.0\% | 17.9\% | 98.7\% | 0.0\% | 100.0\% | 0.0\% | 0.0\% |  |
| AM Peak |  |  |  | 11:00 |  | 11:00 |  |  | 11:00 |
| Volume |  |  |  | 482 |  | 482 |  |  | 482 |
| PM Peak |  |  | 19:00 | 15:00 |  | 15:00 |  |  | 15:00 |
| Volume |  |  | 408 | 617 |  | 617 |  |  | 617 |
| Total | 0 | 0 | 1316 | 7262 | 0 | 7360 | 0 | 0 | 7360 |

ADT Not Calculated

Traffic Data Collection, Inc.
61891 Spring Circle, Washington MI. 48094
Traffic Study Performed For:
Parsons Transportation Group, Inc.

Page 1
Station ID: Southboound RIVERSIDE (M-29)
(Bet. Jay St: \& Vine St.)
SBRiversideN
Site Code: 0001
Date Printed: 26-Jun-03


Traffic Data Collection, Inc.
61891 Spring Circle, Washington MI. 48094
Traffic Study Performed For:
Project: St. Clair
Count Type: 7 Day ATR Volume Count Weather: Clear, Dry Counter\#:8282

## Parsons Transportation Group, Inc.

Page 2
Station ID: Southboound RIVERSIDE (M-29)
(Bet. Jay St. \& Vine St.) SBRiversideN Site Code: 0001
Date Printed: 26-Jun-03 Count By: MGM Pav't: Asphalt 2 Lanes


| \% Avg. <br> WkDay | 101.5\% | 104.7\% | 92,3\% | 0.0\% | 0.0\% | 100.0\% |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| \% Avg. Week | 101.5\% | 104.7\% | 92.3\% | 0.0\% | 0.0\% | 100.0\% | 0.0\% | 0.0\% |  |
| AM Peak | 11:00 | 11:00 | 11:00 |  |  | 11:00 |  |  | 11:00 |
| Volume | 491 | 630 | 485 |  |  | 535 |  |  | 535 |
| PM Peak | 15:00 | 15:00 | 15:00 |  |  | 15:00 |  |  | 15:00 |
| Volume | 666 | 681 | 599 |  |  | 649 |  |  | 649 |
| Total | 8685 | 8952 | 9260 | 7836 | 8030 | 16495 | 6674 | 5977 | 15693 |

ADT Not Calculated


Traffic Data Collection, Inc.
$\begin{array}{lr}\text { Iraffic Data Collection, Inc. } & \text { Station ID: Northbound } \\ 61891 \text { Spring Circle, Washington MI. } 48094 & \text { RIVERSIDE (M-29) } \\ \text { Traffic Study Performed For: } & \text { RE }\end{array}$
Traffic Study Performed For:
Project: St. Clair
Count Type: 7 Day ATR Volume Count
Parsons Transportation Group, Inc.
(Bet. Jay St. \& Vine St.)
NBRiversideN
Site Code: 000003
Date Printed: 26-Jun-03

Count By: MGM Pav't: Asphalt 3 Lanes
Thu

| Start <br> Time | Mon <br> 16-Jun-0 | Tue Wed |
| :---: | :---: | :---: | :---: |


| AM | * | $*$ | * | 67 |
| ---: | :--- | :--- | :--- | :--- |
| $01: 00$ | $*$ | $*$ | $*$ | 35 |

Traffic Data Collection, Inc.
Page 2
61891 Spring Circle, Washington MI. 48094 Traffic Study Performed For:
Project: St. Clair
Count Type: 7 Day ATR Volume Count
Weather: Clear, Dry Counter\#:8222

Parsons Transportation Group, Inc.

Station ID: Northbound RIVERSIDE (M-29)
(Bet. Jay St. \& Vine St.)
NBRiversideN
Site Code: 000003

Date Printed: 26-Jun-03
Week
Average



ADT Not Calculated


## APPENDIX III

TURNING MOVEMENT COUNT DATA

## PARSONS

## 26777 Central Park Blvd., Suite 275 <br> Southfield, MI 48076

File Name : M29ClintonAM
Site Code : 00000000
Start Date : 06/19/2003
Page No : 1
Groups Printed- Unshifted

|  | M 29 <br> From North |  |  |  |  | Sport Bar Driveway From East |  |  |  |  | $\begin{gathered} \text { M } 29 \\ \text { From South } \end{gathered}$ |  |  |  |  | CLINTON From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | $\begin{gathered} \text { Thr } \\ \text { u } \end{gathered}$ | $\begin{gathered} \hline \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{r} \hline \text { Ped } \\ 5 \end{array}$ | App. <br> Total | Left | $\begin{array}{r} \text { Thr } \\ u \\ \hline \end{array}$ | $\begin{aligned} & \text { Rig } \\ & \text { ht } \\ & \hline \end{aligned}$ | $\begin{array}{r} \text { Ped } \\ \hline \end{array}$ | App. <br> Total | Left | $\begin{array}{r} \text { Thr } \\ \text { u } \end{array}$ | $\begin{array}{r} \text { Rig } \\ \text { ht } \end{array}$ | $\begin{array}{r} \text { Ped } \\ \mathrm{s} \end{array}$ | App. Total | Left | $\begin{array}{r} T h r \\ u \\ \hline \end{array}$ | $\begin{aligned} & \text { Rig } \\ & \text { ht } \end{aligned}$ | Ped | App. <br> Total | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.01 |  | 1.0 | $1.0 \mid$ | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 07:00 AM | 1 | 73 | 38 | 0 | 112 | 0 | 0 | 1 | 0 | 1 | 10 | 30 | 0 | 0 | 40 | 11 | 0 | 16 | 0 | 27 | 180 |
| 07:15 AM | 0 | 49 | 41 | 0 | 90 | 1 | 0 | 0 | 0 | 1 | 24 | 67 | 0 | 0 | 91 | 25 | 0 | 10 | 0 | 35 | 217 |
| 07:30 AM | 0 | 54 | 45 | 0 | 99 | 0 | 0 | 0 | 1 | 1 | 30 | 108 | 0 | 0 | 138 | 29 | 0 | 18 | 0 | 47 | 285 |
| 07:45 AM | 1 | 60 | 47 | 0 | 108 | 0 | 0 | 0 | 0 | 0 | 28 | 84 | 0 | 0 | 112 | 31 | 2 | 31 | 0 | 64 | 284 |
| Total | 2 | 236 | 171 | 0 | 409 | $\dagger$ | 0 | , | 1 | 31 | 92 | 289 | 0 | 0 | 381 | 96 | 2 | 75 | 0 | 173 | 966 |


| 08:00 AM | 0 | 69 | 33 | 0 | 102 | 1 | 0 | 0 | 0 | 1 | 14 | 64 | 0 | 0 | 78 | 31 | 1 | 30 | 2 | 64 | 245 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 08:15 AM | 0 | 59 | 23 | 0 | 82 | 0 | 0 | 0 | 0 | 0 | 19 | 65 | 0 | , | 85 | 28 | 0 | 26 | 0 | 54 | 221 |
| 08:30 AM | 1 | 65 | 30 | 3 | 99 | 0 | 0 | 1 | 0 | 1 | 27 | 62 | 0 | 0 | 89 | 27 | 0 | 28 | 0 | 55 | 244 |
| 08:45 AM | 0 | 59 | 50 | 0 | 109 | 0 | 0 | 0 | 0 | 0 | 29 | 63 | 0 | 0 | 92 | 26 | 1 | 25 | 0 | 52 | 253 |
| Total | 1 | 252 | 136 | 3 | 392 | 1 | 0 | 1 | 0 | 2 | 89 | 254 | 0 | 1 | 344 | 112 | 2 | 109 | 2 | 225 | 963 |


| Grand | 3 | 488 | 307 | 3 | 801 | 2 | 0 | 2 | 1 | 5 | 181 | 543 | 0 | 1 | 725 | 208 | 4 | 184 | 2 | 398 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Total | 3 | 1929 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Apprch \% | 0.4 | 60.9 | 38.3 | 0.4 |  | 40.0 | 0.0 | 40.0 | 20.0 |  | 25.0 | 74.9 | 0.0 | 0.1 |  | 52.3 | 1.0 | 46.2 | 0.5 |  |
| Total \% | 0.2 | 25.3 | 15.9 | 0.2 | 41.5 | 0.1 | 0.0 | 0.1 | 0.1 | 0.3 | 9.4 | 28.1 | 0.0 | 0.1 | 37.6 | 10.8 | 0.2 | 9.5 | 0.1 | 20.6 |

## PARSONS

## 26777 Central Park Blvd., Suite 275 <br> Southfield, MI 48076

File Name : M29ClintonPM
Site Code : 00000000
Start Date: 06/19/2003
Page No : 1

|  | $\begin{gathered} \text { M } 29 \\ \text { From North } \end{gathered}$ |  |  |  |  | Sport Bar Driveway From East |  |  |  |  | $\begin{gathered} \text { M } 29 \\ \text { From South } \end{gathered}$ |  |  |  |  | CLINTON From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | $\begin{array}{r} \text { Thr } \\ u \\ \hline \end{array}$ | $\begin{array}{r} \mathrm{Rig} \\ \mathrm{ht} \end{array}$ | $\begin{array}{r} \text { Ped } \\ 5 \end{array}$ | App. <br> Total | Left | $\begin{array}{r} T h r \\ u \\ \hline \end{array}$ | $\begin{array}{r} \text { Rig } \\ \text { ht } \\ \hline \end{array}$ | $\begin{array}{r} \text { Ped } \\ s \\ \hline \end{array}$ | App. Total | Left | $\begin{gathered} \text { Thr } \\ u \end{gathered}$ | $\begin{gathered} \text { Rig } \\ \mathrm{ht} \end{gathered}$ | Ped | App. Total | Left | $\begin{gathered} \text { Thr } \\ u \end{gathered}$ | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | Ped s | App. Total | $\begin{gathered} \text { Int } \\ \text { Total } \end{gathered}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.01 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 03:00 PM | 2 | 91 | 69 | 2. | 164 | 3 | 0 | 10 | 0 | 13 | 18 | 102 | 0 | 0 | 120 | 32 | 2 | 29 | 0 | 63 | 360 |
| 03:15 PM | 2 | 108 | 45 | 0 | 155 | , | 1 | 6 | 0 | 8 | 42 | 104 | 2 | 1 | 149 | 41 | 0 | 31 | 4 | 76 | 388 |
| 03:30 PM | 0 | 133 | 86 | 1 | 220 | 2 | 0 | 5 | 0 | . 7 | 64 | 120 | 0 | 0 | 184 | 69 | 0 | 39 | 1 | 109 | 520 |
| 03:45 PM | 0 | 48 | 27 | 0 | 75 | 1 | 0 | 6 | 0 | 7 | 39 | 100 | 0 | 0 | 139 | 31 | 2 | 43 | 2 | 78 | 299 |
| Total | 4 | 380 | 227 | 3 | 614 | 7 | 1 | 27 | 0 | 35 | 163 | 426 | 2 | 1 | 592 | 173 | 4 | 142 | 7 | 326 | 1567 |


| 04:00 PM | 0 | 97 | 60 | 0 | 157 | 1 | 0 | 2 | 0 | 3 | 42 | 107 |  | 1 | 151 | 63 | 2 | 48 | 3 |  | 427 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:15 PM | 2 | 114 | 51 | 0 | 167 | 2 | 0 | 2 | 0 | 4 | 30 | 98 | 1 | 1 | 130 | 51 | 1 | 32 | 2 | 86 | 387 |
| 04:30 PM | 3 | 120 | 62 | 0 | 185 | 0 | 0 | 0 | 0 | 0 | 31 | 112 | 1 | 0 | 144 | 44 | 1 | 38 | 1 | 84 | 413 |
| 04:45 PM | 3 | 115 | 61 | 0 | 179 | 0 | 0 | 3 | 0 | 3 | 32 | 109 | 3 | 0 | 144 | 58 | 1 | 44 | 2 | 105 | 431 |
| -Total | 8 | 446 | 234 | 0 | 688 | 3 | 0 | 7 | 0 | 10 | 135 | 426 | 6 | 2 | 569 | 216 | 5 | 162 | 8 | 391 | 1658 |


| Grand | 12 | 826 | 461 | 3 | 1302 | 10 | 1 | 34 | 0 | 45 | 298 | 852 | 8 | 3 | 1161 | 389 | 9 | 304 | 15 | 717 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Total | 3225 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Apprch \% | 0.9 | 63.4 | 35.4 | 0.2 |  | 22.2 | 2.2 | 75.6 | 0.0 |  | 25.7 | 73.4 | 0.7 | 0.3 |  | 54.3 | 1.3 | 42.4 | 2.1 |  |
| Total \% | 0.4 | 25.6 | 14.3 | 0.1 | 40.4 | 0.3 | 0.0 | 1.1 | 0.0 | 1.4 | 9.2 | 26.4 | 0.2 | 0.1 | 36.0 | 12.1 | 0.3 | 9.4 | 0.5 | 22.2 |

APPENDIX IV
EXISTING CAPACITY ANALYSIS WORKSHEETS
LG/LG
Phase I' | Phase 2
Phase I' | Phase 2

$\left|\begin{array}{l|l|}\mathrm{G} / \mathrm{C}=0.525 & \mathrm{G} / \mathrm{C}=0.322 \\ \mathrm{G}=31.5^{\prime \prime} & \mathrm{G}=19.3^{\prime \prime} \\ \mathrm{Y}+\mathrm{R}=4.5^{\prime \prime} & \mathrm{Y}+\mathrm{R}=4.7 \mathrm{n} \\ \mathrm{OFF}=0.0 \% & \mathrm{OFF}=60.0 \%\end{array}\right|$

```
    C=60 sec G= 50.8 sec = 84.7%% Y= 9.2 sec = 15.3% Ped= 0.0 sec = 0.0%
```



N Approach
4.5 A


| RT | 11/1 | 0.208 | 1.000 | 1326 | 1326 | 189 | 0.143 | 0.2 | A |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| TH | 22/2 | 0.115 | 0.525 | 1775 | 1779 | 267 | 0.150 | 7.5 | A |  |

## S Approach

8.4 A



W Approach
15.6 B

| RT | 11/1 | 0.116 | 0.322 | 412 | 488 | 89 | 0.182 | 15.5 | B | 57 £t |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LT | 11/1 | 0.120 | 0.322 | 468 | 545 | 111 | 0.204 | 15.6 | B | 71 ft |

Traffic Analysis Report - M-29 Corridor - Saint Clair, MI
Existing 2003 Volumes \& Conditions - PM Peak

SIGNAL2000/TEAPAC[Ver 1.01.00] - Capacity Analysis Summary
Intersection Averages for Int \# 0-M-29 \& Clinton
Degree of Saturation (v/c) 0.37 Vehicle Delay 10.3 Level of Service B+


| Eane Group | Width/ Lanes | Reqd | ${ }^{\text {C Used }}$ | Servi | ) Rat | : Adj | v/c | HCM | L | Queue Model 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| N Approach |  |  |  |  |  |  |  | 5.6 A |  |  |
| RT | 11/1 | 0.256 | 1.000 | 1326 | 1326 | 256 | 0.193 | 0.3 | A | 13 ft |
| TH | 22/2 | 0.181 | 0.525 | 1775 | 1779 | 500 | 0.281 | 8.3 | A | 127 ft |

$$
\text { S Approach } \quad 10.2 \quad \mathrm{~B}+
$$

W Approach 18.5 B

| RT | 11/1 | 0.185 | 0.322 | 412 | 488 | 189 | 0.387 | 18.1 | B | 126 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LT | 11/1 | 0.200 | 0.322 | 468 | 545 | 244 | 0.448 | 18.8 | B | 163 ft |

## APPENDIX V MDOT M-29 SPEED STUDY DATA

,



APPENDIX VI FUTURE CAPACITY ANALYSIS WORKSHEETS

Future 2025 Volumes \& No Build Condition-AM Peak

SIGNAL2000/TEAPAC[Ver 1.01.00] - Capacity Analysis Summary
Intersection Averages for Int \# 0-M-29 \& Clinton Degree of Saturation ( $v / c$ ) 0.28 Vehicle Delay 8.7 Level of Service $A$


| Lane Group | $\left\|\begin{array}{r} \text { Width } / \\ \text { Lanes } \end{array}\right\|$ | $\begin{aligned} & \mathrm{G} / \mathrm{C} \\ & \text { Reqd } \cdot \text { Used } \end{aligned}$ | Service Rate Adj <br> @C (vph) @E : Volume | $\mathrm{v} / \mathrm{c}$ | $\begin{gathered} \text { HCM } \\ \text { Delay } \end{gathered}$ | $\begin{aligned} & \mathrm{L} \\ & \mathrm{~S} \end{aligned}$ | Queue Model 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |



S Approach

9.3 A



W Approach 16.3 B


| RT | 11/1 | 0.140 | 0.322 | 412 | 488 | 122 | 0.250 | 16.2 | B | 79 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LT | 11/1 | 0.141 | 0.322 | 468 | 545 | 144 | 0.264 | 16.3 | B | 93 ft |

Future 2025 Volumes \& No Build Condition-PM Peak

SIGNAL2000/TEAPAC[Ver 1.01.00] - Capacity Analysis Summary
Intersection Averages for Int \# 0-M-29 \& Clinton
Degree of Saturation (v/c) 0.51 Vehicle Delay 12.4 Level of Service B+

$\mathrm{C}=60 \mathrm{sec} \quad \mathrm{G}=50.8 \mathrm{sec}=84.7 \%{ }^{\circ} \mathrm{Y}=9.2 \mathrm{sec}=15.3 \%$ Ped= $0.0 \mathrm{sec}=0.0 \%$


N Approach 6.1 A

| RT | 11/1 | 0.310 | 1.000 | 1326 | 1326 | 333 | 0.251 | 0.5 | A | 18 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| TH | 22/2 | 0.224 | 0.525 | 1775 | 1779 | 656 | 0.369 | 9.0 | A | 173 |

S Approach 13.9 B+


W Approach
$21.0 \mathrm{C}+$


| RT | $\mathrm{II} / 1$ | 0.220 | 0.322 | 412 | 488 | 244 | 0.500 | 20.1 | $\mathrm{C}+$ | 168 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| IT | ft |  |  |  |  |  |  |  |  |  |
| LT | $11 / 1$ | 0.243 | 0.322 | 468 | 545 | 322 | 0.591 | 21.7 | $\mathrm{C}+\mid$ | 225 |
| ft |  |  |  |  |  |  |  |  |  |  |

Future 2025 Volumes \& Alternative 1-AM Peak

SIGNAL2000/TEAPAC[Ver 1.01.001 - Capacity Analysis Summary
Intersection Averages for Int \# 0-M-29 \& Clinton Degree of Saturation ( $v / c$ ) 0.22 Vehicle Delay
8.3 Level of Service A

## Sq 11



LG/LG

$\left|\begin{array}{l|l|}\mathrm{G} / \mathrm{C}=0.525 & \mathrm{G} / \mathrm{C}=0.322 \\ \mathrm{G}=31.5^{\prime \prime} & \mathrm{G}=19.3^{\prime \prime} \\ \mathrm{Y}+\mathrm{R}=4.5^{\prime \prime} & \mathrm{Y}+\mathrm{R}=4.7^{\prime \prime} \\ \mathrm{OFF}=0.0 \% & \mathrm{OFF}=60.0 \%\end{array}\right|$
$C=60 \mathrm{sec} G=50.8 \mathrm{sec}=84.7 \%$ Y $\quad Y=9.2 \mathrm{sec}=15.3 \%$ Ped= $0.0 \mathrm{sec}=0.0 \%$

| Lane Group | Width/ | .Reqd | C Used | Service. Rate @C: (viph) @E |  | Adj Volume | v/c | HCM Delay | I Queue <br> S Model <br> I  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| N Approach |  |  |  |  |  |  |  | 4.7 A |  |  |
| RT | 11/1 | 0.220 | 1.000 | 1516 | 1516 | 244 | 0.161 | 0.2 | A | 11 ft |
| TH | 22/2 | 0.141 | 0.525 | 1775 | 1779 | 356 | 0.200 | 7.8 | A | 88 ft |



## S Approach

8.2 A


W Approach 16.3 B

| RT | 11/1 | 0.140 | 0.322 | 412 | 488 | 122 | 0.250 | 16.2 | B | 79 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| IT | 11/1 | 0.141 | 0.322 | 468 | 545 | 144 | 0.264 | 16.3 | B | 93 ft |

```
Sq 11 | Phase 1 | | Phase 2 |
|l|l
C=60 sec G= 50.8 sec = 84.7% Y= 9.2 sec = 15.3% Ped= 0.0 sec = 0.0%
```



## N Approach

6.1 A

| RT | 11/1 | 0.275 | 1.000 | 1516 | 1516 | 333 | 0.220 | 0.3 | A | 16 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| TH | 22/2 | 0.224 | 0.525 | 1775 | 1779 | 656 | 0.369 | 9.0 | A | 173 ft |

S Approach
$10.4 \mathrm{~B}+$

| TH | $24 / 2$ $12 / 1$ | 0.211 0.381 | 0.525 0.525 | 1839 312 | 1840 362 | 633 200 | 0.344 0.552 | 8.8 15.5 | * A | 164 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| W Approach |  |  |  |  |  |  |  | 21.0 | C+ |  |
| RT LT | $11 / 1$ $11 / 1$ | 0.220 0.243 | 0.322 0.322 | 412 468 | 488 545 | 244 322 | 0.500 0.591 | 20.1 21.7 | C+ C+ | $\begin{array}{lll}168 & \text { ft } \\ 225 & \text { ft }\end{array}$ |



$\left|\begin{array}{l|l|}\mathrm{G} / \mathrm{C}=0.525 & \mathrm{G} / \mathrm{C}=0.322 \\ \mathrm{G}=31.5^{\prime \prime} & \mathrm{G}=19.3^{\prime \prime} \\ \mathrm{Y}+\mathrm{R}=4.5^{\prime \prime} & \mathrm{Y}+\mathrm{R}=4.7 \mathrm{\prime} \mathrm{\prime} \\ \mathrm{OFF}=0.0 \% & \mathrm{OFF}=60.0 \%\end{array}\right|$
$\mathrm{C}=60 \mathrm{sec} \quad \mathrm{G}=50.8 \mathrm{sec}=84.7 \% \mathrm{Y}=9.2 \mathrm{sec}=15.3 \% \mathrm{Ped}=0.0 \mathrm{sec}=0.0 \%$


$$
\mathrm{N} \text { Approach } 8.8 \text { A }
$$


S Approach 10.3 B+

| TH | 12/1 | 0.276 | 0.525 | 933 | 968 | 422 | 0.436 | 10.2 | * $\mathrm{B}+$ | 213 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LT | 12/1 | 0.271 | 0.525 | 341 | 391 | 133 | 0.340 | 10.6 | * B + | 72 ft |

W Approach 16.3 B

| RT | 11/1 | 0.140 | 0.322 | 412 | 488 | 122 | 0.250 | 16.2 | B | 79 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LT | 11/1 | 0.141 | 0.322 | 468 | 545 | 144 | 0.264 | 16.3 | B. | 93 ft |

Traffic Analysis Report - M-29 Corridor - Saint Clair, MI

Intersection Averages for Int \# $0-\mathrm{M}-29^{\circ} \&$ Clinton Degree of Saturation (v/c) 0.62 Vehicle Delay 17.9 Level of Service $B$

| Sq 11 | Phase 1 | Phase 2 |
| :---: | :---: | :---: |
| LG/LG |  |  |
|  | + + |  |
|  | + + |  |
| 111 | <+ + |  |
|  | v | ヘ |
|  | ^ | ++++ |
| North | <+ * |  |
|  | + * | ++++ |
|  | + * | V |
|  | $\mathrm{G} / \mathrm{C}=0.525$ | $\mathrm{G} / \mathrm{C}=0.322$ |
|  | $\mathrm{G}=31.5^{\prime \prime}$ | $\mathrm{G}=19.3^{\prime \prime}$ |
|  | $Y+R=4.5^{\prime \prime}$ | $\mathrm{Y}+\mathrm{R}=4.7{ }^{\prime \prime}$ |
|  | OFF $=0.0 \%$ | $\mathrm{OFF}=60.0 \%$ |

$\mathrm{C}=60 \mathrm{sec} \quad \mathrm{G}=50.8 \mathrm{sec}=84.7 \% \mathrm{Y}=9.2 \mathrm{sec}=15.3 \%$ Ped= $0.0 \mathrm{sec}=0.0 \%$

| Group | Width/. Lanes | Reqd Us:ed | Service Rate @்C. (vph). @E | Adj <br> Volume | v/c | HCM <br> Del:ay | S. | Queue <br> Model 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |

## N Approach

## 11.0 <br> B+



## S Approach

24.1 C+


| TH | $12 / 1$ | 0.379 | 0.525 | 933 | 968 | 633 | 0.654 | 13.7 | $* \mathrm{~B}+$ | 368 ft |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| LT | $12 / 1$ | 0.590 | 0.525 | 169 | 217 | 200 | 0.922 | 56.7 | *E+ | 211 ft |

## W Approach

$21.0 \quad \mathrm{C}+$

| $\begin{aligned} & \mathrm{RT} \\ & \mathrm{LT} \end{aligned}$ | $11 / 1$ $11 / 1$ | $\begin{aligned} & 0.220 \\ & 0.243 \end{aligned}$ | $\begin{aligned} & 0.322 \\ & 0.322 \end{aligned}$ | $\begin{aligned} & 412 \\ & 468 \end{aligned}$ | $\begin{aligned} & 488 \\ & 545 \end{aligned}$ | $\begin{aligned} & 244 \\ & 322 \end{aligned}$ | $\begin{aligned} & 0.500 . \\ & 0.591 \end{aligned}$ | $\begin{aligned} & 20.1 \\ & 21.7 \end{aligned}$ | $\mathrm{C}+$ $\mathrm{C}+$ + | 168  <br> 225 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |

Traffic. Analysis Report - M-29 Corridor - Saint Clair, MI
Future 2025 Volumes \& Alternative 3-AM Peak

SIGNAL2000/TEAPAC[Ver 1.01.00] - Capacity Analysis Summary
Intersection Averages for Int \# 0-M-29 \& Clinton
Degree of Saturation (v/c) 0.36 Vehicle Delay 10.8 Level of Service B+

$84.7 \% \mathrm{Y}=9.2 \mathrm{sec}=15.3 \%$ Ped= $0.0 \mathrm{sec}=0.0 \%$


N Approach
8.8 A


| $\mid \mathrm{RT}+\mathrm{TH}$ | 24/2 | \|0.214 | 0.525 | 1722 | 1728 | 600 | 0.347 | 8.8 | A | 157 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 5 Approach |  |  |  |  |  |  |  | $10.3 \mathrm{~B}+$ |  |  |
| TH | 12/I | 0.276 | 0.525 | 933 | 968 | 422 | 0.436 | 10.2 | * B+ | 213 ft |
| LT | 12/1 | 0.271 | 0.525 | 341 | 391 | 133 | 0.340 | 10.6 | *B+1 | 72 ft |

W Approach - 16.3 B

| RT | 11/1 | 0.140 | 0.322 | 412 | 488 | 122 | 0.250 | 16.2 | B | 79 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LT | 11/1 | 0.141 | 0.322 | 468 | 545 | 144 | 0.264 | 16.3 | B | 93 ft |

Intersection Averages for Int \# 0-M-29 \& Clinton
Degree of Saturation ( $v / c$ ) 0.62 Vehicle Delay 17.9 Level of Service $B$


| Lane Group | Width. Lanes | $\mid \text { Reqd }$ | /C Used | $\begin{aligned} & \text { Sèrv } \\ & \text { @C: } \end{aligned}$ |  | Adj <br> Volume | v/c | HCM Delay | L | $\left\|\begin{array}{cc}\text { Queue } \\ \text { Model } & 1\end{array}\right\|$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| N Approach |  |  |  |  |  |  |  | 11.0 B+ |  |  |
| \| RT+TH | 24/2 | 0.319 | \| 0.525 | 1742 | 1747 | 989 | 0.566 | 11.0 | B+ | 294 ft\| |
| S Approach |  |  |  |  |  |  |  | 24.1 C+ |  |  |
| TH | 12/1 | 0.379 | 0.525 | 933 | 968 | 633 | 0.654 | 13.7 | * B+ | 368 ft |
| LT | 12/1 | 0.590 | 0.525 | 169 | 217 | 200 | 0.922 | 56.7 | *E+ | 211 ft |

$$
\text { W Approach } \quad 21.0 \quad \mathrm{C}+
$$



| RT | $11 / 1$ | 0.220 | 0.322 | 412 | 488 | 244 | 0.500 | 20.1 | $\mathrm{C}+$ | 168 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| LT | $11 / 1$ | 0.243 | 0.322 | 468 | 545 | 322 | 0.591 | 21.7 | $\mathrm{C}+$ | 225 |
| Lt |  |  |  |  |  |  |  |  |  |  |

## APPENDIX VII

 TRAFFIC CRASH DATA


| Control Section | Mile <br> Point | PR <br> Number | Mile Point | Crash Type | Veh1 |  |  | Veh 2 |  |  | Fatal | Injury | Crash |  |  | Crash <br> Report | Reel | Frame |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Dir | Intent | Impact | Dir | Intent |  |  |  |  |  |  |  |  |  |
| 77052 | 7.830966704 |  | 7.31 RE-LT |  | N | GO ST | FRONT | N | TNLT | Impact |  |  | Day | Date | Hour |  |  |  |
|  |  |  | N | GOT | FRONT | N | TNLT | REAR |  | 1 | MON | 09/18/2000 | 4PM-5PM |  |  |  |  |  |
| 77052 | 7.83 | 0966704 |  |  | 7.30 | RE-ST | $N$ | GO ST | FRONT-L | N | STOP RD | REAR-R |  |  |  | - |  | 0214677 | 529 | 1544 |
| 77052 | 7.84 | 0966704 | 7 | RE-DR | S |  |  | N | STOP RD | REAR-R |  |  | SAT | 01/10/1998 | 10AM-11AM | 1434599 | 188 | 6761 |
| 77052 |  | - |  |  | S | GO ST | FRONT | S | STOP RD | REAR |  |  | WED | 04/14/1999 | 7AM-8AM |  |  |  |
|  | 7.85 | 0966704 | 7.33 | RE-DR | S | S/STOP RD | NONE | S | S/STOP R |  |  |  |  | 04/14/1999 | 7AM-8AM | 7443996 | 246 | 5885 |
| 77052 |  | 0 |  |  |  |  |  | S | S/STOPR | REAR |  |  | THU | 07/23/1998 | 11AM-NOON | 7444051 | 211 | 5157 |
|  | 7.87 |  |  | M | S | GO ST | FRONT-R |  | PASSING | ERROR |  |  | SUN | 10/31/1999 |  |  |  |  |
| 77052 | 7.92 | 0966704 | 7.40 | SS-SM | N | CHG LN | SIDE-R | N |  |  |  |  |  | 10/31/1999 | 3PM-4PM | 4861794 | 271 | 1025 |
| 77052 | 8.08 | 0966704 | 7.56 |  |  |  | SIDE-R | N | ERROR | FRONT-L |  |  | SUN | 11/14/1999 | 7AM-BAM | 9433845 | 271 | 1023 |
|  |  | 09667 | 7.56 | MSC-SN( | S | GO ST | UNDER |  |  |  |  |  | THU |  |  |  | 27 | 1023 |
| 77052 | 8.08 | 0966704 | 7.56 | MSC-ML7 | S | GO ST |  |  |  |  |  |  | THU | 01/20/2000 | 7AM-8AM | 0214378 | 504 | 4701 |
| 77052 | 8.11 |  |  |  |  |  |  |  | CHG LN | SIDE-R |  |  | WED | 10/28/1998 | BAM-9AM |  |  |  |
|  |  | 0966704 | 7.59 | MSC-SNC | E | GO ST | FRONT |  |  |  |  |  |  |  | BAM-GAM | 7444106 | 221 | 8587 |
| 77052 | 8.13 | 0966704 | 7.61 | FXOBJ | N |  | SIDE-R |  |  |  |  |  | SUN | 03/08/1998 | 5AM-6AM | 1434594 | 196 | 9669 |
| 77052 | 8.13 |  |  |  |  |  | S.DE-R |  |  |  |  |  | FRI | 02/06/1998 | 1AM-2AM |  |  |  |
|  |  | 0966704 | 7.61 | MSC-SNC | S | GO ST | FRONT-R |  |  |  |  |  |  | 02106/1998 | - ${ }^{\text {a }}$ | 4910356 | 194 | 1081 |
| 77052 | 8.13 | 0966704 | 7.61 | RE-ST | S |  |  |  |  |  |  |  | SAT | 08/28/2000 | 5AM-6AM | 0214487 | 525 | 0880 |
| 77052 |  |  | 7.61 | RE-S | s | CHG LN | FRONT | S | S/STOP RD | REAR |  |  | MON | 12/14/1998 | $3 P M-4 P M$ |  |  |  |
|  | 8.15 | 0966704 | 7.63 | FXOBJ | N | GO ST | FRONT |  |  |  |  |  |  |  | $3 \mathrm{~N}-4 \mathrm{PM}$ | 7444136 | 228 | 4822 |
| 77052 | 8.16 | 0966704 | 7.64 | FXOBJ | S |  | REAR |  | , |  |  |  | FRI | 12/18/1998 | 7AM-8AM | 1434738 | 228 | 4826 |
| 77052 | 8.23 | 0967105 | 1.06 | RE-LT | N | TNLT | REAR-R |  |  |  |  |  | THU | 04/17/1997 | 2AM-3AM | 4910533 | 153 | 1618 |
| 77052 | 8.25 | - | 1.06 | RE-LT | N | TNLT | REAR | N | GO ST | FRONT |  |  | MON | 06/01/1998 | 4PM-5PM |  |  |  |
|  |  | 0967105 | 1.08 | MSC-MLT | $N$ | STOP RD | REAR-L |  |  |  |  |  |  | 06101/98 | 4PM-5PM | 4862544 | 208 | 0199 |
| 77052 | 8.25 | 0967105 | 1.08 | RE-DR | S | S |  |  |  |  |  | 1 | WED | 04/14/1999 | 10AM-11AM | 7443998 | 245 | 5184 |
|  |  | 096705 | . 0 | RE-DR | S | S | FRON | 5 | TNRT | REAR | , |  | TUE | 08/18/1998 | 10AM-11AM |  |  |  |
| 77052 | 8.25 | 0967105 | 1.07 | PRKNG | $N$ | LV PRK | FRONT-R | S | GO ST | FRONT-R |  |  |  |  |  | 7444054 | 213 | 6820 |
| 77052 | 8.26 | 0967105 | 1.09 | AN-DR | E |  |  |  |  | FRONT-R |  | 1 | SAT | 02/21/1998 | 6PM-7PM | 1434653 | 194 | 1077 |
|  |  | , | 1.09 | AN-DR | E | TNLT | FRONT-L | S | GOST | REAR-R |  | 1 | WED | 06/28/2000 | 1PM-2PM |  |  |  |
| 77052 | 8.26 | 0967105 | 1.08 | RE-DR | S | GO ST | FRONT |  | S/STOP RD | REAR |  |  |  | 06/28/2000 | (PM-2FM | 0214506 | 519 | 3010 |
|  |  | 09 |  |  | E |  |  |  |  | rear |  | 1 | WED | 09/15/1999 | 6AM-TAM | 9433791 | 263 | 8553 |
| $77052$ | 8.26 |  | 1. | A | $E$ | START R | FRONT | S | GOST | REAR-R |  |  | TUE | 07/11/2000 | NOON- |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | NOON-1P | 0214285 | 522 | 2164 |



| Crashes Selected: |
| :--- |
| InterChange |
| Intersection |
| Segment |

Time Period: 01/01/1997 thru 12/31/2000 (4 Years)
Segment
77052 7.13-9.99


Crashes Selected:
InterChange
Intersection
Segment
Time Period: 01/01/1997 thru 12/31/2000 (4 Years)
Location:

| Control Section | Mile Point | PR <br> Number | Mile Point | Crash Type | Veh1 |  |  | Veh 2 |  |  | Fatal | Injury |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Dir | Intent | Impact |  |  |  | Crash |  | Crash Report | Reel | Frame |
| 77052 | 8.54 | 0967105 | 1.36 | HD-LT | S | GOST | FRONT-R | N | TNLT | Impact |  |  |  |  |  | Day | Date | Hour |
| 77052 | 8.55 | 0967105 | 1.38 | PRKNG | N | LV PRK |  |  |  |  |  |  | TUE |  | 05/20/1997 | 2PM-3PM | 4910517 | 157 | 0524 |
|  |  |  |  | PRNG | N | LVPRK | FRONT-R | N | GOST | FRONT |  |  | SUN | 12/20/1998 | 6PM-7PM | 1434704 | 229 |  |
| 77052 | 8.55 | 0967105 | 1.38 | RE-DR | S | GO ST | FRONT-R | S | STOP RD | REAR-L |  |  | TUE | 05/26/1998 |  |  |  | 9778 |  |
| 77052 | 8.55 | 0967105 | 1.38 | RE-DR | S | S/STOP RD | FRONT-R |  |  |  |  |  |  |  | 8AM-9AM | 4910371 | 208 | 0196 |  |
| 77052 | 8.55 | 096710 |  |  |  | Start ro | FRONT-R | S | PASSING | REAR-L |  |  | THU | 05/14/1998 | 5PM-6PM | 1434707 | 208 | 0195 |  |
|  |  | 096710 | 1.37 | RE-ST | S | START RD | FRONT | S | S/STOP RD | REAR |  |  |  | 05/10/1997 |  |  |  |  |  |
| 77052 | 8.58 | 0967105 | 1.40 | AN-ST | S | GO ST | SIDE-R | E | TNRT |  |  |  | SAT |  | 5PM-6PM | 4910515 | 157 | 0517 |  |
| 77052 | 8.62 | 0967105 | 1.45 | HD-LT | S | GOST |  |  | TNuT | FRONT |  |  | FRI | 04/23/1999 | 2PM-3PM | 7444021 | 245 | 5183 |  |
|  |  |  |  |  |  |  | FRONT | W | TNLT | SIDE-R |  | 2 | THU | 11/27/1997. | 10PM-11PM | 1434605 |  |  |  |
| 77052 | 8.65 | $096710{ }^{\circ}$ | 1.47 | DU-LT | N | TNLT | SIDE-L | N | TNLT | REAR-R |  |  |  |  |  |  | 179 | 9349 |  |
| 77052 | 8.66 | 0967105 | 1.48 | RE-ST | N | TNLT |  |  |  | AR-R |  |  | FRI | 07/24/1998 | 3PM-4PM | 4862550 | 213 | 6825 |  |
|  |  |  |  |  | N | TNLT | REAR | N | GO ST | FRONT-L |  |  | THU | 07/23/1998 | 6PM-7PM | 7443964 |  |  |  |
| 77052 | 8.67 | 0967105 | 1.49 | RE-ST | S | GOST | FRONT | S | TNLT | REAR |  |  |  | 07123/498 |  |  | 211 | 5163 |  |
| 77052 | 8.70 | 0967105 | 1.52 | RE-ST | N | GOST |  |  |  |  |  |  | SAT | 09/30/2000 | 6PM-7PM | 0214679 | 528 | 0129 |  |
|  |  |  |  |  |  | GOST | FRONT | N | S/STOP RD | REAR |  |  | SAT | 11/06/1999 | NOON-1PM | 7444059 | 271 |  |  |
| 77052 | 8.70 | 0967105 | 1.52 | OT-DR | E | ENT RD | FRONT | N | GO ST |  |  |  |  |  |  |  |  | 1027 |  |
| 77052 | 8.71 | 0967105 | 1.53 | HD-ON | S | GO ST |  | N | GOST | NT-L |  | 1 | THU | 10/02/1997 | 10AM-11AM | 1434666 | 174 | 9728 |  |
| 77052 |  |  |  |  | S | GOST | FRONT | N | GO ST | FRONT |  | 2 | TUE | 09/15/1998 | 7AM-8AM | 7444080 | 217 |  |  |
|  | 8.71 | 0967105 | 1.53 | RE-DR | N | GO ST | REAR | N | GO ST | FRONT |  |  | MON | 08/03/1998 |  |  |  | 3719 |  |
| 77052 | 8.72 | 0967105 | 1.54 | AN-TN | W | TNLT | SIDE-R | S | GO ST |  |  |  |  |  | 4PM-5PM | 7443969 | 213 | 6691 |  |
| 77052 | 8 |  |  |  |  |  |  |  | GO ST | FRONT-L |  |  | THU | 06/10/1999 | 11AM-NOON |  | 250 | 6154 |  |
|  |  | 0967105 | 1.54 | RE-DR | N | GO ST | FRONT-R | N | STOP RD | REAR-L |  |  |  |  |  | 9433728 |  |  |  |
| 77052 | 8.72 | 0967105 | 1.54 | AN-ST | S | GO ST | FRONT-R |  |  |  |  |  | FRI | 01/08/1999 | $3 P M-4 P M$ | 7444153 | 235 | 0291 |  |
|  |  |  |  |  |  |  | FRONT-R | E | TNLT | FRONT |  | 1 | WED | 07/23/1997 | 3PM-4PM | 4861785 | 165 |  |  |
| 77052 | 8.74 | 0967105 | 1.56 | AN-ST | $N$ | PARKED | REAR-L | N | TNLT | SIDE-R |  |  |  |  |  |  |  | 9879 |  |
| 77052 | 8.74 | 0967105 | 1.56 | RE-DR |  |  |  |  |  | SIDE-R |  |  | tue | 09/19/2000 | $3 P M-4 P M$ | 0214455 | 529 | 1553 |  |
|  |  |  |  |  |  | S/STOP RD | FRONT-L | N | ERROR | ERROR |  |  | FRI |  |  |  |  |  |  |
| 77052 | 8.83 | 0967105 | 1.66 | MSC-MLT |  | GO ST |  | S | PARKED |  |  |  |  | 09/24/1999 | 9AM-10AM | 9433815 | 263 | 8549 |  |
| 77052 | 885 | 09671 |  |  |  |  |  |  | PARKED | SIDE-L |  |  | THU | 08/17/2000 | 3PM-4PM |  |  |  |  |
| 77052 | 8.85 | 0967105 | 1.67 | SS-SM | NW | UTURN | FRONT-L | N | GO ST | FRONT-R |  |  |  |  |  | 0214315 | 525 | 0874 |  |
|  |  |  |  |  |  |  |  |  |  |  |  | 1 | SAT | 10/03/1998 | 8PM-9PM | 7444038 | 221 | 8603 |  |





| Crashes Selected InterChange Intersection Segment | Time Period: <br> Location: <br> 77052 7.13-9.99 |  | $01 / 01 / 1997$ | thr | 12/31/2000 |  | (4 Years) |  | 06/17/2003 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach Direction | Total | Head Number of Crashes By Type |  |  |  |  |  |  |  |  |  |
|  |  | Head On | Head On-Lt | Angle Str | Angle Turn | Rear End | Rear Turn | Ped | Park | Drive | Other |
|  | 26 | 0 | 1 | 8 | 5 | 1 | 0 | 0 | 0 |  | Other |
| E | 7 | 0 | 0 | 2 | 1 | 0 |  |  |  | 9 | 2 |
| N | 63 | 0 | 2 | 2 | 2 | 0 | 3 | 0 | 1 | 2 | 1 |
| NE | 1 | 0 | 0 | 0 | 2 | 20 | 3 | 0 | 2 | 16 | 16 |
| NW | 2 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| S | 50 | 2 | 0 | , | 0 | 0 | 0 | 0 | 1 | 0 | 1 |
| W | 9 | 0 | 1 | 3 | 1 | 7 | 1 | 0 | 0 | 14 | 22 |
| W | 5 | 0 | , | 1 | 3 | 0 | 0 | 0 | 1 | 3 | 0 |
| Other |  | - | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 2 | 1 |
| Total |  |  | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 2 | 2 |
| Average Per year | 169 42.3 | 2 0.5 | - ${ }^{4}$ |  | 15 | 29 | 4 | 1 | 5 | 48 | 45 |
| Percent of Total | 100.0 | 0.5 1.2 | -1.0 2.4 | 4.0 | 3.8 | 7.3 | 1.0 | 0.3 | 1.3 | 12.0 | 11.3 |
| 1997 |  |  |  | 9.5 | 8.9 | 17.2 | 2.4 | 0.6 | 3.0 | 28.4 | 26.6 |
| 1998 | 52 | 1 | 2 |  | 4 | 5 | 0 | 0 | 0 | 12 | 7 |
| 1999 | 47 | 1 | 0 | 6 | 2 | 9 | 2 | 1 | 3 | 14 | 14 |
| 2000 |  |  | 1 | 4 | 6 | 8 | 0 | 0 | 1 | 15 | 11 |
|  |  |  | 1 | 4 | 3 | 7 | 2 | 0 | 1 | 7 | 13 |

Crashes Selected Time Period: 01/01/1997 thru 12/31/2000 (4 Years)

InterChange Intersection Segment

## TYPE OF CRASH

| YEAR | REAR-END LEFT TURN | HEAD-ON <br> LEFT TURN | REAR-END | ANGLE | SIDESWIPE | PEDES. TRIAN | HEAD-ON | DRIVERELATED | $\begin{aligned} & \text { FIXED } \\ & \text { OBJECT } \end{aligned}$ | OTHERS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1997\| | 0 0\% | $26 \%$ | $516 \%$ | 6 19\%\| | $3 \quad 9 \%$ | $0 \quad 0 \%$ | 0 0\% | $1238 \%$ | $2 \%$ | 2 6\% |
| 1998 | 2 4\% | 0 0\% | $917 \%$ | $8 \quad 15 \%$ | 5 5 10\%\| | 1 2\% | $12 \% \mid$ | $14.27 \% \mid$ | 3 6\% | $917 \%$ |
| 1999 | $0 \quad 0 \%$ | $12 \%$ | 8 8 17\% | 10 21\%\| | 4 ¢\% | - 0\%\| | 1 2\% | 15 32\%\| | 3 6\% | $511 \%$ |
| 20001 | 2 5\% | $13 \%$ | 7 18\% | $718 \%$ | $13 \%$ | 0 0\% | $0 \quad 0 \% \mid$ | $718 \%$ | 2 5\% | 11 29\% |
| Total | 4 2\% | 4 2\% | 29 17\% | $31 \quad 18 \%$ | 13 8\% | 1 1\% | $21 \%$ | 48 28\% | 10 6\% | 27 16\% |

## PAVEMENT CONDITION

| YEAR | DEBRIS |  | DRY |  |  | ICY |  | MUDDY |  | SLUSHY |  |  | SNOWY |  | WET |  | OTHERS |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1997 | 0 | 0\% |  | 25 | 78\% | 0 |  | 0 |  |  | 0 |  | - 1 |  | 6 | 19\% | 0 | 0\% |
| 1998 | 0 | 0\% |  | 42 | 81\% | 1 | 2\% | 0 |  |  | 0 |  | 2 | 4\% | 7 | 13\% | 0 |  |
| 1999 | 0 | 0\% |  | 33 | 70\% | 3 | 6\% | 0 | 0\% |  | 0 | 0\% | 6 | 13\%\| | 5 | 11\% | 0 | 0\% |
| 2000 | 0 | 0\% |  | 27 | 71\% | 0 | 0\% | 0 | 0\% |  | 0 | 0\% | 3 | 8\%\| | 8 | 21\% | 0 | 0\% |
| Total | 0 | 0\% | - | 127 | 75\% | 4 | 2\% | 0 | 0\% |  | 0 | 0\% | 12 | 7\% |  | 15\% | 0 | 0\% |

## LIGHT CONDITION



| YEAR | FATAL CRASHES | NUMBER KILLED | INJURY CRASHES | NUMBER <br> INJURED | PROPERTY DAMAGE | OTHERS | TOTAL GRASHES | TOTAL INJURIES |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1997 | 0 0\% | 0 | 14 44\% | 21 | 18 56\% | - 0\% | 32 | 21 |
| 1998 | 0 0\% | 0 | 12 23\% | 17 | $4077 \%$ | 0 0\% | 52 | 17 |
| \|1999| | 0 0\% | 0 | 18 38\% | 29 | $2962 \%$ | 0 0\% | 47 | 29 |
| 2000 | 0 0\% | 0 | 14 37\% | 17 | 24 63\% | $0 \quad 0 \%$ | 38 | 17 |
| Total | 0 | 0 | 58 | 84 | 111 | 0 | 169 | 84 |


| Crashes Selected <br> InterChange | Time Period: $01 / 01 / 1997$ <br> Location: | thru $12 / 31 / 2000$ | (4 Years) |
| :--- | :--- | :--- | :--- |
| Intersection |  |  |  |
| Segment |  |  |  |$\quad 770527.13-9.99$|  |
| :--- |



## DAY

| SUNDAY | 0 | 6 | 11 | 17 | 10.06 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| MONDAY | 0 | 7 | 13 | 20 | 11.83 |
| TUESDAY | 0 | 7 | 14 | $21 \mid$ | 12.43 |
| WEDNESDAY | 0 | 16 | 12 | 28 | 16.57 |
| THURSDAY | 0 | 10 | 28 | 38 | 22.49 |
| FRIDAY | 0 | 6 | 22 | 28 | 16.57 |
| SATURDAY | 0 | 6 | 11 | 17 | 10.06 |
| UNK | 0 | 0 | 0 | 0. | 0.00 |
| TOTAL | 0 | 58 | 111 | 169 | 100.01 |

## LIGHT

| UNCODE / ERROR | 0 | 0 | 0 | 0 | 0.00 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| DAYLIGHT | 0 | 42 | 82 | 124 | 73.37 |
| DAWN | 0 | 1 | 3 | 4 | 2.37 |
| DUSK | 0 | 0 | 4 | 4 | 2.37 |
| DARK LIGHTED | 0 | 13 | 20 | 33 | 19.53 |
| DARK UNLIGHTED | 0 | 2 | 4 | 2.37 |  |
| OTHER | 0 | 0 | 0 | 0.00 |  |
| TOTAL | 0 | 58 | 111 | 169 | 100.01 |


| Crashes Selected <br> InterChange | Time Period: $01 / 01 / 1997$ <br> Intersection | Location: |  |  |
| :--- | :--- | :--- | :--- | :--- |
| Segment | $72 / 31 / 2000$ (4 Years) |  |  |  |



## WEATHER

| ERROR | 0 | 1 | 1 | 2 | 1.18 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| CLEAR | 0 | 30 | 52 | 82 | 48.52 |
| Cloudy | 0 | 21 | 40 | 61 | 36.09 |
| FOG / SMOKE | 0 | 0 | 0 | 0 | 0.00 |
| RAIN | 0 | 4 | 6 | 10 | 5.92 |
| SNOWIBLOW | 0 | 2 | 11 | 13 | 7.69 |
| SEVERE WND | 0 | 0 | 0 | 0 | 0.00 |
| SLEET/HAIL | 0 | 0 | 0 | 0 | 0.00 |
| OTHER | 0 | 0 | 1 | 1 | 0.59 |
| TOTAL | 0 | 58 | 111 | 169 | 99.99 |


| Crashes Selected <br> InterChange | Time Period: 01/01/1997 thru 12/31/2000 (4 Years) <br> Intersection <br> Segmention: |  | $770527.13-9.99$ |
| :--- | :--- | :--- | :--- |


| SURFACE | FATAL | INJURY | P. DAMAGE | \# CRASHES | PERCENT |
| :---: | :---: | :---: | :---: | :---: | :---: |
| ERROR | 0 | 0 | 0 | 0 | 0.00 |
| DRY | 0 | 44 | 83 | 127 | 75.15 |
| WET | 0 | 11 | 15 | 26 | 15.38 |
| ICY | 0 | 1 | 31 | 4 | 2.37 |
| SNOWY | 0 | 2 | $10 \mid$ | 12 | 7.10 |
| MUDDY | 0 | 0 | 01 | 0 | 0.00 |
| SLUSHY | 0 | 0 | 01 | 0 | 0.00 |
| DEBRIS | 0 | 0 | 0 | 0 | 0.00 |
| OTHER | 01 | 0 | 0 | 01 | 0.00 |
| TOTAL | 0 | 58 | 111 | 169 | 100.00 |

GONDITION

| IN CNST ZONE | 0 | 0 | 1 | $1\|r\| r\|r\|$ | 0.59 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| IN UTIL ZONE | 0 | 0 | 0 | $0 \mid$ | 0.00 |
| OTHER | 0 | 58 | 110 | 168 | 99.41 |
| TOTAL | 0 | 58 | 111 | 169 | 100.00 |

HWY TYPE .

| INTERSECTION | 0 | 47 | 88 | 135 | 79.88 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| MID-BLOCK | 0 | 11 | 23 | $34 \mid$ | 20.12 |
| NON-TRAFFIC | 0 | 0 | 0 | 0 | 0.00 |
| TOTAL | 0 | 58 | 111 | 169 | 100.00 |

RELATIONSHIP TO ROAD

| ERROR | 0 | 0 | 0 | 0 | 0.00 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| IN GORE | 0 | 0 | 0 | 0 | 0.00 |
| IN MEDIAN | 0 | 0 | 0 | 0 | 0.00 |
| ON ROAD | 0 | $56 \mid$ | 105 | $161 \mid$ | 95.27 |
| ON SHOULDR | 0 | 0 | $2 \mid$ | $2 \mid$ | 1.18 |
| OUTSD SHDR | 0 | $1 \mid$ | $3 \mid$ | $4 \mid$ | 2.37 |
| UNKNOWN | $0 \mid$ | $1 \mid$ | $1 \mid$ | $2 \mid$ | 1.18 |
| TOTAL | 0 | 58 | 111 | 169 | 100.00 |

Crashes Selected Time Period: 01/01/1997 thru 12/31/2000 (4 Years)
InterChange Location:
Intersection
Segment
77052 7.13-9.99


TOTAL LANES

| 1 | 0 | 1 | 1 | 2 | 1.18 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 2 | 0 | 9 | 17 | 26 | 15.38 |
| 3 | 0 | 2 | 5 | 7 | 4.14 |
| 4 | 0 | 45 | 87 | 132 | 78.11 |
| 5 | 0 | 0 | 0 | 0 | 0.00 |
| 6 | 0 | 1 | 1 | 2 | 1.18 |
| 7 | 0 | 0 | 0 | 0 | 0.00 |
| 8 | 0 | 0 | 0 | 0 | 0.00 |
| 9 | 0 | 0 | 0 | 0 | 0.00 |
| 99 | 0 | 0 | 0 | 0 | 0.00 |
| TOTAL | 0 | 58 | 111 | 169 | 99.99 |

## Public Comments

# M-29 CORRIDOR PLANNING AND RESEARCH <br> Public Information Meeting Summary <br> City of St. Clair - City Hall <br> November 10, 2004 

Meeting attendees were asked to fill out a brief survey that included a list of eight issues. Attendees were asked to rank these issues from 1 to 8 in the order of importance to them, 1 being the most important and 8 being the least important. The following rankings list the overall opinion of the entire group.

| 1 | Improve safety at pedestrian crossings |
| :--- | :--- |
| 2 | Encourage motorists to obey posted speeds |
| 3 | Improve turn movements, especially at Clinton Street |
| 4 | Improve aesthetics and suggest way-finding signage; strategic sing placement |
| 5 | Reduce roadway noise |
| 6 | Provide a continuous non-motorized path along the M-29 Corridor |
| 7 | Provide adequate parking on-street |
| 8 | Maintain existing traffic flow rate (level of service) |

-Objectives ranked in order of importance; based on 41 surveys received November 10, 2004
Attendees were also provided with space for comments and observations. The following is a listing of the comments written by these attendees.

I cannot tell you how many times I cross that street in the course of one day! I'm sorry but this issue appears to be so ridiculous. The road goes through a city, so why would you raise the speed? What about traffic flow for our businesses? If you eliminate traffic, you eliminate clients and customers.

There is currently no safe way for residents in my area to ride a bike or walk to downtown St. Clair from our neighborhood. There are no bike paths or sidewalks.

I would prefer Alternative 2 or 3 for the downtown area, and agree with the proposed plans north and south.

Possibly eliminate the in/out driveways at the mall parking lot. Make entry only on Jay/Vine or Second. With all the parking available in the mall, there are plenty of places to park. Prefer Alternative 3 without parking spaces.

Enforce current speed limits. We approve north of downtown plan.
As the community grows in size and more people are coming to the downtown area, is having only 1 lane in each direction going to provide enough driving space to not discourage people coming due to congestion? Having a sidewalk along Palmer Park is absolutely necessary, but not necessarily as part of the "Bay and Bridge" plan.

Speeds leaving town south and north increases above posted speed limits. Speed limits should reflect pedestrian safety.

North Riverside Proposal is a good idea. We like the decrease of 3 lanes. East side of Riverside is very difficult to cross over 4 lanes of traffic with small children on bikes. Need a bike path/sidewalk.

Like the idea of Burm downtown and also bike paths and parking. Slowing downtown traffic is main concern!

Traffic flows alright now. Traffic is backed up to our house now, when the bridge is up. It will take 2-3 changes in the lights to clear traffic. With the four lanes in front of our office, it gives us the option of waiting in the center and cars will move to the other lane to give us even more time.

Love the North Riverside proposal! Alternate 1 is good balance between parking, amenities.

We live within city limits. When we exit or enter our front door, the first thing we view is a 50 MPH sign, which is within city limits. Cars are traveling at a rate of 60 MPH due to lack of ticketing. Truck noise is too much.

Need more crosswalks with signals.
Speed limit should now be lowered to 25 MPH and enforced-this would make it safe. We need downtown more parking friendly for pedestrians who come for our events. We need more parking for business fronts and park.

I think this is a stupid idea. Leave it the way it is. I'm against any change to M-29.
Slow down traffic and enforce speed limits. Enforce noise ordinances.
Very disappointed in plan, especially south side. Thought plan would involve a "major rebuilding of M-29." Any future plans must include placing all above ground utilities underground especially on South Oakland. Plan will not solve many of the "identified problematic issues."

Prefer Alternateive 2.

## Prefer Alternative 1.

Changing the state highway 29 to 3 lanes increases congestion, makes passing impossible. What about garbage pick-up and school bus stops? More accidents. A step
backward. Place signs and lights telling cars when they can exit restaurant parking lots. Do no obstruct traffic flow with parked cars.

Have an adequate sidewalk now, waste of money to provide more. I'm also a bike rider and have no problem. This is a state highway—slowing traffic is no solution. As now, provide parking only in commercial and park area. Signs are always helpful. Raise gas to 4 bucks a gallon. Seriously, maybe people will change to smaller cars. Population growth, more cars, everybody in a hurry. Reducing to three lanes from four is a step backwards. Traffic flow people are long against it. Middle lane is a suicide lane. See: old Gratiot, was three now two. Boulevard? Two thumbs DOWN! Like many, this is an old community with outmoded roads and other utilities and we have to live with some inconvenience and change slowly. Taxes are high and can be better spent on healthcare reform and aid for the poor. $\$ 100,000$ for this study seems largely a waste to me.

Have lighting on both sides of the street.
I believe we mostly have adequate on-street parking. Noise is a problem for us, but if traffic is reduced to 2 lanes, we will never be able to get out of our driveways. People usually do a rolling stop or do not stop at Brown onto M-29, which does not give us easy exit from our driveways. We sit and sit, usually I will just edge out onto M-29 into the closest lane south and they will go around or into passing lane. Forget getting onto M29 and going north from our house! Maybe better enforcement would slow traffic? I really like the first alternate for downtown and I like the idea of bike lanes on the north side of town, but what can be done about the Brown St. turning issue?

Get ride of all new "No Parking" signs and reduce number of speed limit signs. I prefer Alternative 1, but have a paved 9 or 10 foot path next to the road on the east side.

I like north and south alternatives and I like Alternative 1 for downtown. I would like to minimize any problems that could arise at the St. Clair Inn due to going from 2 lanes to boulevard.

I am leaning towards Alternative 1. I like the idea of a decorative landscape median which would provide some safety crossing the 4 lanes and it would enhance St. Clair. I would also be for the decorative median north of town.

MDOT has trashed N. Riverside with a tremendous amount of signs. I suggest eliminating all signs except 40 MPH in city limits, where speed changes. Install "quaint" street lights through city on M-29.

I think the only issue that needs to be addressed are speed limits and enforcement!!!
Three lanes starting north of Vine St. will cause a problem for traffic getting in and out of the St. Clair Inn, because we are just north of Vine Street.

Walk-ability and pedestrian safety are important, as well as reducing traffic speed. Both could be accomplished with these options. I prefer Alternative 1, but feel Alternative 3 would be more practical for St. Clair. ( $7{ }^{\text {th }}$ Street)

Design must provide a connection between commercial area and residential neighborhoods with the park and riverfront. Non-motorized traffic should be encourages and protected with the appropriate design.

I really liked Alternative 3.
Eliminate commercial traffic on Vine and Brown!
I am more concerned with traffic (large trucks) coming down Vine.
Narrow the internal freeway!

Cost Estimates of Roadway Alternatives





[^0]:    PROPOSED M-29 TYPICAL CROSS-SECTION
    (ALTERNATE 3)

[^1]:    * A volume-to-capacity ratio $=1.0$ is reached at different densities depending upon the free-flow speed on each segment.

    SOURCE: Transportation Research Board, 2000 Highway Capacity Manual.

